Longitudinal and transverse dispersion coefficients in braided rivers
Li Gu, Zinan Jiao and Zulin Hua

ABSTRACT

The dispersion characteristics of braided rivers are presently unclear. The comprehensive flow structure in a physical braided river model was measured and was used to estimate its dispersion coefficient tensor. The largest values of the longitudinal and transverse dispersion coefficients occurred in the separation zone in two anabranches. The separation zone disappeared in a small diversion angle model of braided rivers where the coefficients were smaller. As for the sectional transverse distribution, the two coefficients varied markedly and an interesting negative correlation between them appeared in several sections. The dispersion coefficients increased with upstream flow rates. Comparison between the coefficients for different anabranch widths revealed higher values in wider sections. Finally, the values of the laboratory tests were compared with those in a real braided river, and relatively larger coefficients were found in natural rivers. The findings of this paper could be helpful in understanding the dispersion characteristics and in estimating pollutant concentration in braided rivers.

Key words | braided river, dispersion coefficients, secondary flow, separation zone, vertical velocity distribution

INTRODUCTION

The dispersion coefficient is an important parameter representing dispersion characteristics of a river, and plays a great role in estimating pollutant concentration successfully. Substantial efforts have been made by Fischer et al. in the past few decades to estimate longitudinal and transverse dispersion coefficients in a regular straight channel. The dimensionless transverse mixing coefficient in rectangular channels varied from 0.1 to 0.2 (Fischer et al. 1979). The value was 0.17 in a straight section of the river Rhine near Karlsruhe measured by Webel & Schatzmann (1984). As to the dimensionless longitudinal dispersion coefficient, its value was equal to 5.93 for a vertical logarithmic velocity profile in an infinitely wide channel (Elder 1959). In general, the coefficient in natural straight rivers was much greater than Elder’s result. Seo & Cheong (1998) pointed out that the coefficient was 395.11 in the Mississippi River. Greater longitudinal dispersion coefficients were also estimated by Yotsukura et al. (1970) and Fuat Toprak & Savci (2007).

Based on the research of regular straight channels, the effects of more factors such as bed roughness, cross-sectional shape, curvature, and vegetation in natural channels on dispersion were taken into account (Fukuoka & Sayre 1975; Hou & Christensen 1976; Deng et al. 2001; Perucca et al. 2009). High bed surface roughness was found to increase the gradient in vertical velocity profiles and, thus, shear dispersion (Schulz et al. 2012). An empirical formula considering the radius of curvature and overall bend length was proposed by Fukuoka & Sayre (1975) for meandering rivers. In the river with riparian vegetation, a quite sparse riparian vegetation can affect longitudinal dispersion by 70–100% with respect to the case without vegetation (Perucca et al. 2009).

However, the dispersion coefficients may not be homogeneous along the natural river because of the heterogeneous flow characteristics (Piasecki & Katopodes 1999). The space variation of dispersion coefficients has been observed in the meandering channel. Sayre & Yeh (1973) reported that in a meandering channel transverse mixing coefficients vary from one-half its average value in the upstream portion of a bend to twice the average in the downstream portion. In sinuous open channels, the maximum transverse dispersion coefficients are reached...
downstream of the bend apex in the regions of strong secondary flow, and minimum values in the straighter regions (Boxall & Guymer 2005). A similar phenomenon was also reported in the North Saskatchewan River (Dow et al. 2009). Furthermore, in the section of the meander cycle running from apex to cross-over, longitudinal dispersion coefficients were lower than the apex to apex values, whilst the cross-over to apex reach had higher values (Boxall & Guymer 2007).

Spatial variation of dispersion coefficient in meandering channels is mainly caused by their complicated flow structure, and especially for the transverse dispersion the secondary currents play a great role. Compared with mean-flow, and especially for the transverse dispersion the spatially heterogeneous distribution, which is more attractive to researchers. However, few studies on dispersion in braided rivers have been reported so far, and their dispersion characteristics are unclear, especially the spatial distribution of longitudinal and transverse dispersion coefficients.

The present paper extends the previous work on dispersion to the braided river, and detailed information of spatial distribution of dispersion coefficients in braided rivers was first analyzed. Laboratory tests in the braided river model were conducted, and the components of the dispersion tensor were calculated using velocity profiles. Dispersion characteristics for various flow rates, different anabranch widths, and different diversion angles were compared. The relationships between dispersion properties and cross-sectional secondary flow were further analyzed. Moreover, the influence of a separation zone is also discussed. Finally, the dispersion coefficients in the anabranch of the braided Brahmaputra River were calculated, and the relevance of the laboratory to the natural braided river are discussed.

**MATHEMATICAL METHOD**

Following the definition of the one-dimensional longitudinal dispersion coefficient, the equation for the dispersion tensor was given by Fischer (1978):

\[ M_x = -h \left( D_{xx} \frac{\partial c}{\partial x} + D_{xy} \frac{\partial c}{\partial y} \right) \] (1a)

Then the Cartesian components of the dispersion tensor are given:

\[ D_{xx} = -\frac{1}{h} \int_0^h u' \int_0^e \frac{1}{e} \int_0^c u' \, dz \, dz \, dz \] (2a)

\[ D_{xy} = -\frac{1}{h} \int_0^h v' \int_0^e \frac{1}{e} \int_0^c u' \, dz \, dz \, dz \] (2b)

\[ D_{yx} = -\frac{1}{h} \int_0^h v' \int_0^e \frac{1}{e} \int_0^c v' \, dz \, dz \, dz \] (2c)

\[ D_{yy} = -\frac{1}{h} \int_0^h u' \int_0^e \frac{1}{e} \int_0^c v' \, dz \, dz \, dz \] (2d)

where \( M_x \) and \( M_y \) denote unit width mass flux in \( x \) and \( y \) direction, \( \langle c \rangle \) is the depth average of \( c \), \( h \) is the local depth of flow, \( u' \) and \( v' \) denote vertical deviations of point velocities with respect to depth-averaged velocities \( u \) and \( v \) respectively, \( z \) is the vertical coordinate, and \( \epsilon \) is a vertical turbulent diffusion coefficient. Therefore, measured velocity data can be used to calculate the Cartesian components of the dispersion tensor.

The longitudinal and transverse dispersion coefficients along the streamline and normal to the streamline are defined as \( D_L \) and \( D_T \). The Cartesian components of the dispersion coefficient tensor \( (D_{xx}, D_{xy}, D_{yx}, D_{yy}) \) are related to \( D_L \) and \( D_T \) (Alavian 1986):

\[ D_{xx} = D_L \frac{u^2}{U^2} + D_T \frac{v^2}{U^2} \] (3a)

\[ D_{xy} = D_{yx} = (D_L - D_T) \frac{uv}{U^2} \] (3b)

\[ D_{yy} = D_T \frac{u^2}{U^2} + D_L \frac{v^2}{U^2} \] (3c)

By relating Equation (2) to Equation (3), the longitudinal and transverse dispersion coefficients \( D_L \) and \( D_T \) can be estimated from the calculated components of the dispersion coefficient tensor. The calculated values of \( D_{xy} \) and \( D_{yx} \) obtained using vertical profile data have almost the same values, and therefore these two terms are averaged to be
used in Equation (4) (Seo et al. 2008):

\[
\begin{pmatrix}
    D_L \\
    D_T
\end{pmatrix} = (A^T A)^{-1} A^T \begin{pmatrix}
    D_{xx} \\
    D_{yx} = D_{xy}
\end{pmatrix}
\]

(4)

where

\[
A = \begin{bmatrix}
    u^2 & v^2 \\
    uu & Uv & Uv & Uv \\
    uu & Uv & Uv & Uv \\
    v^2 & u^2 & U^2 & U^2
\end{bmatrix}
\]

is a matrix and \( U = \sqrt{u^2 + v^2} \).

The bicubic interpolation method is used to calculate the values of the dispersion coefficient tensor. The velocity profile within the boundary layer changes dramatically, while the accuracy of measured velocities 0.5 cm from the bottom can be confirmable. The velocity profile was assumed to be decreased linearly to zero at the bottom of channel for simplicity. Thus, relative results can be obtained by numerical integration of Equation (2).

**EXPERIMENTS**

To study dispersion properties along anabranches, a laboratory physical model of the braided river was constructed. The flume was made of organic glass and was 25 m long. The width of the upstream main channel, left anabranch and right anabranch were denoted as \( B_0, B_1, \) and \( B_2 \), respectively (Figure 1(a)). The parameter \( B^* = B_1 / (B_1 + B_2) \), which was the ratio of the width of the left anabranch to the sum widths of the two anabranches, was introduced. \( B_0 \) was equal to 0.5 m, and \( B_1 + B_2 = 0.7 \) m. The mid-bar could be moved to change the width of the two anabranches. The inner bank was defined as the wall close to the mid-bar and the outer bank was located on the other side. Different width ratios of the two anabranches and different upstream flow rates were considered in the experiments, as listed in Table 1.

![Diagram of the experimental model](image_url)

**Figure 1** | Diagram of the experimental model (a), and location diagrams of measured sections with (b) case 1 with \( B^* = 0.5 \), (c) case 3 with \( B^* = 0.33 \) and (d) case 4 with \( B^* = 0.5 \) (length unit: mm).

**Table 1** | Experimental flow conditions

<table>
<thead>
<tr>
<th>Case</th>
<th>Diversion angle (°)</th>
<th>( B^* )</th>
<th>Discharge of inflow ( Q_o ) (L/s)</th>
<th>Flow depth ( H_o ) (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>86</td>
<td>0.5</td>
<td>5.96 (Run F-1), 7.51 (Run F-2), 9.69 (Run F-3), 11.18 (Run F-4), 13.41 (Run F-5)</td>
<td>0.149</td>
</tr>
<tr>
<td>2</td>
<td>86</td>
<td>0.41</td>
<td>7.51</td>
<td>0.149</td>
</tr>
<tr>
<td>3</td>
<td>86</td>
<td>0.33</td>
<td>7.51</td>
<td>0.149</td>
</tr>
<tr>
<td>4</td>
<td>29</td>
<td>0.50</td>
<td>8.04</td>
<td>0.139</td>
</tr>
</tbody>
</table>
In addition, two diversion angles were used to investigate the effect of the separation zone on the dispersion.

The velocity data were measured by a three-component Sontek ADV. Ten sections were set up in each anabranch for cases 1 to 3, and nine sections were used in case 4 (Figures 1(b)–1(d)). For convenience of analysis in the following experiments, S1, S2, ... , S10 were assigned as the names of sections in the anabranch, whether in the left or the right anabranch. In each cross-section, seven to nine verticals were measured, with six to eight test points along each vertical. The position of verticals for measuring velocity are shown in Table 2.

ESTIMATION OF DISPERSION COEFFICIENTS IN ANABRANCHES

The dimensionless values of $D_L/\mu u^*$ and $D_T/\mu u^*$ were calculated and analyzed in the braided river models with different width ratios, upstream flow rates and diversion angles. Shear velocity is estimated by the following equation

$$u^* = (gRS)^{1/2},$$

where $g$ is the acceleration of gravity; $R$ is the hydraulic radius; $S_f = n^2U^2R^{-4/3}$ is the friction slope; and $n$ is Manning coefficient. In the analysis of the longitudinal variation along the anabranch, the section-averaged $D_L$ and $D_T$ were used.

Dispersion in anabranches with the separation zone

Contours for the longitudinal and transverse dispersion coefficients in case 1 are shown in Figures 2(a) and 2(b) respectively. It is obvious that $D_L/\mu u^*$ and $D_T/\mu u^*$ both have relatively large values near the outer bank in the first half of the anabranch where the separation zone occurs. Higher values of $D_T/\mu u^*$ also appear near the inner bank, which has relatively high values of stream-wise velocity in the first few sections. In the second half of the anabranches, from section S7 to S10, dispersion coefficients become smaller and vary less. The velocity fields at the surface and the bottom for case 1 are depicted in Figures 2(e) and 2(f) respectively. It is apparent that the velocity varies widely between the bottom and the surface in the separation zone, which contributes to larger values of the dispersion coefficients. This increase is consistent with the features observed in the stream with the ‘dead zones’ (Valentine & Wood 1977). Velocity differences between the bottom and the surface were small in the second half of the anabranches, leading to relatively smaller coefficient values. Section L7 and L9 were chosen to investigate the influence of secondary flow on the transverse dispersion coefficients. In section L7 of case 1, the variations of $D_L/\mu u^*$ and $D_T/\mu u^*$ exhibited a similar pattern in the transverse direction (Figure 2(i)). The reason was that the center of the secondary flow cell in section 7 had a stronger velocity deviation and its location was close to the main stream-wise flow. Thus a similar one-peak distribution of $D_L/\mu u^*$ and $D_T/\mu u^*$ was caused. An interesting negative correlation was found in section L9 (Figure 2(j)). It is clear that there are two cells of secondary flow with opposite directions of rotation in this section (Figure 2(h)). Each of the cells is skewed to one side wall, leading to two peaks of $D_L/\mu u^*$, and the main flow is located in the center of the cross-section which is between two cells. All these factors contribute to the negative correlation of $D_L/\mu u^*$ and $D_T/\mu u^*$.

The longitudinal variation of the section-averaged dispersion coefficients in case 1 is shown in Figures 2(c) and 2(d). Due to the symmetrical arrangement of the two anabranches, the left anabranch was chosen for detailed analysis. $D_L/\mu u^*$ and $D_T/\mu u^*$ had similar variation trends along the anabranch. They both exhibited the unimodal distribution pattern, with the highest value occurring in section L3. The values of $D_L/\mu u^*$ varied between 2.17 and 7.79 in sections S1 to S6 and between 0.4 and 1.5 in sections S7 to S10. Figure 2(e) shows that a separation zone appeared from S2 to S5, which dramatically enhanced the dispersion ability. Therefore, the average values of $D_T/\mu u^*$ also

**Table 2 | Position of verticals for measuring velocity**

<table>
<thead>
<tr>
<th>Case</th>
<th>Number of verticals (anabranch-1)</th>
<th>Number of verticals (anabranch-2)</th>
<th>Distance from the outer bank (cm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>7</td>
<td>7</td>
<td>5, 9, 13, 17.5, 22, 26, 30</td>
</tr>
<tr>
<td>2</td>
<td>6</td>
<td>8</td>
<td>4.5, 8.5, 12.5, 16.5, 20.5, 24.5</td>
</tr>
<tr>
<td>3</td>
<td>5</td>
<td>9</td>
<td>4.5, 8, 12, 16, 19.5</td>
</tr>
<tr>
<td>4</td>
<td>7</td>
<td>7</td>
<td>5, 9, 13, 17.5, 22, 26, 30</td>
</tr>
</tbody>
</table>

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increased in the first half of the anabranch, ranging from 1.16 to 7.16. The average value of $D_T/\nu u^*$ for sections far from the separation zone, including sections S8, S9, and S10, varied from 0.34 to 0.62, which is comparable to the results of Holley & Abraham (1976) for a curved channel. Rutherford (1994) also reported that $D_T/\nu u^*$ varied from 0.3 to 0.9 in meandering natural channels and ranged from 1 to 3 in curved channels with a strong secondary flow. In the lower half of the anabranch, the results of a braided river were similar to the values for natural curved channels.

Contours for dispersion coefficients in cases 2 and 3 are shown in Figures 3(a)–3(d). Obviously, the existence of the separation zone still led to larger values of dispersion coefficients in the first half of the anabranches. Compared with case 2, the zone of high dispersion coefficients in case 3 was larger. The reason for this was that larger separation-zone areas and higher velocity magnitudes existed in the wider right anabranch of case 3. In case 1, with $B^*=0.5$, the dispersion coefficients did not change in the confluence. However, in cases 2 and 3, with two asymmetric anabranches, the dispersions were obviously enhanced in the confluence. There was a negative correlation between dispersion in the confluence and the width ratio $B^*$. The non-uniform velocity distribution caused by the flow mixing from two asymmetric anabranches with different flow rates might have been the reason for this. A few interesting negative values can be found in the separation zone, which were caused by measured negative velocity values. This is a unique phenomenon which can be found only in the separation zone.

In cases 2 and 3, the variations of section-averaged $D_L/\nu u^*$ and $D_T/\nu u^*$ in the left and right anabranches are shown in Figures 3(e)–3(h). It can be concluded that the values of $D_L/\nu u^*$ and $D_T/\nu u^*$ generally increased with increasing anabranch width, and that longitudinal variation had a similar trend to case 1, which means that the highest value appeared in the sections in the separation zone in all cases. The value of $D_T/\nu u^*$ varied from 0.26 to 5.5 for the left anabranch and from 0.41 to 7.10 for the right anabranch in case 2 (from section S1 to S10). In case 3, $D_T/\nu u^*$ ranged from 0.19 to 3.07 in the left anabranch and from 0.77 to 7.64 in the right anabranch (from section S1 to S10).

Furthermore, the dispersion coefficients in the five cases with different flow rates ((Run F-1 to Run F-5, in Table 1) were calculated. In the left anabranch, both $D_L$ and $D_T$ increased as the flow rate $Q$ increased. The results for the
experimental data in braided rivers are consistent with earlier studies in single rivers (Elder 1959; Fischer et al. 1979).

**Dispersion in anabranches without a separation zone**

Contours for dispersion coefficients in case 4 without a separation zone are depicted in Figures 4(a) and 4(b). Higher values were found in the entrance and exit of the anabranches, which was different from rivers with a separation zone. The minimum value appeared near the apex of each anabranch. Compared with case 1, the dispersion coefficients were relatively smaller in case 4. Whether or not the separation zone existed, higher values could also be found at the head of the mid-bar.

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**Figure 3** | $D_L/hu^*$ and $D_T/hu^*$ in cases 2 and 3: (a) and (b) contour of $D_L/hu^*$ in case 2 and 3, (c) and (d) contour of $D_T/hu^*$ in cases 2 and 3, (e) and (f) section-averaged $D_L/hu^*$ of the left and right anabranch, (g) and (h) section-averaged $D_T/hu^*$ of the left and right anabranch.

**Figure 4** | $D_L/hu^*$ and $D_T/hu^*$ and flow structure in case 4: (a) contour of $D_L/hu^*$, (b) contour of $D_T/hu^*$, (c) longitudinal variation of section-averaged $D_L/hu^*$, (d) longitudinal variation of section-averaged $D_T/hu^*$, (e)–(g) secondary current in L2, L4, and L7.
occurred because a relatively lower velocity value appeared at the bottom of the inner bank near the head of the mid-bar, leading to a stronger velocity deviation.

The longitudinal variation of the dimensionless section-averaged dispersion coefficients is also shown in Figures 4(c) and 4(d) and has a different trend from case 1. The highest value of $D_L/hu^*$ (5.12) was found in the entrance of the anabranch, and the value decreased along the anabranch until section L4, after which the value was basically stable. The highest value of $D_L/hu^*$ (7.79) in case 1 was almost 1.5 times the highest value (5.12) in case 4.

The minimum value of $D_T/hu^*$ (0.06) occurred in section L4 (Figure 4(d)). As shown in Figures 4(e)–4(g), it is interesting to note that the helical direction of the secondary flow was reversed after section L4. The magnitude of the transverse velocities was relatively smaller here. Therefore, the dimensionless value of $D_T/hu^*$ in section L4 was smallest. However, the existence of one secondary flow cell in sections L2 and L7 led to a higher value of $D_T/hu^*$. The value of $D_T/hu^*$ in case 4 varied from 0.06 to 0.36, which was smaller than the value in case 1. The results for a braided river with small diversion angle were similar to these values, ranging from 0.09 to 0.24 in the laboratory rectangular channel investigated by Okoye (1970). It can be concluded that the values of $D_L/hu^*$ and $D_T/hu^*$ decrease as the diversion angle decreases.

The transverse variation of the dispersion coefficients in the separation zone of case 1 was compared with that in case 4. As shown in Figure 5, section L3 was chosen for comparison. The value of $D_L/hu^*$ was much higher near the outer bank, where a separation zone occurred, but its value decreased quickly outside the separation zone in section L3. An obvious negative correlation between $D_L/hu^*$ and $D_T/hu^*$ can be seen in section L3 for case 1. By comparison, the values of $D_L/hu^*$ and $D_T/hu^*$ in case 4 were smaller and varied little over the section where there is no separation zone.

**Correlation between longitudinal and transverse dispersion**

In the low range of the transverse dispersion coefficient, an exponential increase in the longitudinal dispersion coefficient was found by Boxall & Guymer (2007) as the transverse dispersion coefficient decreased. In the anabranch of braided rivers, there was no obvious relationship in the separation zone, where both $D_L/hu^*$ and $D_T/hu^*$ were extremely large. However, in the lower half of the anabranch without a separation zone, an obvious negative correlation between the distributions of longitudinal and transverse dispersion coefficients existed (Figure 6), where the longitudinal dispersion coefficient was increasing when the transverse dispersion coefficient was decreasing.

**DISCUSSION**

In order to reveal the correlation between the values obtained in laboratory experiments and a natural river, dispersion in a braided channel of the Brahmaputra River in Bangladesh is discussed further. Velocity profile in the anabranch was measured by Richardson et al. (1996), and its dispersion coefficients were calculated by using Equation (4). The transverse variation of $D_L/hu^*$ and $D_T/hu^*$ at the entrance section of the left anabranch (May 1994 survey) are presented in Figure 7. The maximum value of

![Figure 5](https://i.imgur.com/5Q5Q5Q5.png)  
**Figure 5** | Transverse variation of $D/hu^*$ in section L3 for cases 4 and 1: (a) $D_L/hu^*$, (b) $D_T/hu^*$.

![Figure 6](https://i.imgur.com/6Q6Q6Q6.png)  
**Figure 6** | Dispersion coefficients in cases 2 and 3 (S7-S10): (a) $D_L/hu^*$, (b) $D_T/hu^*$.

![Figure 7](https://i.imgur.com/7Q7Q7Q7.png)  
**Figure 7** | Transverse variation of dimensionless dispersion coefficients $D/hu^*$. 

$D_L/hu^*$ happened above the talweg where the primary isovels displayed a single core of maximum velocity (Figure 7(a)). Compared with the entrance section 3 in case 1 with a large diversion angle, a roughly similar transverse variation could be found (Figure 5(a)). Its maximum value of $D_L/hu^*$ also occurs at the location of the primary flow. According to the analysis of Richardson et al. (1996), strong secondary flow was found on the left side of the channel (250 m to 600 m) and in the right side of the channel (850 m to 1044 m). Thus higher dispersion coefficients were observed at the two places and two peak values could be seen in Figure 7(b). The same conclusion was drawn before in the laboratory tests and the Figure 5(b) shows two similar peaks of $D_L/hu^*$ in two sides of the channel for case 1. Differently, Figure 7(b) shows a third peak of higher values between 600 to 700 m where there is a biggest flow depth. In general, higher magnitude of values was found in real rivers. $D_L/hu^*$ varies from 2.55 to 302.74 and $D_T/hu^*$ varies from 0.45 to 65.41. This is mainly because of the higher speed in the natural rivers compared with that in the laboratory channel. Furthermore, an irregular section shape and bed surface roughness are also responsible for this in natural rivers.

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