

Impact of an incomplete solute mixing model on sensor network design

Pedro Romero-Gomez, Kevin E. Lansey and Christopher Y. Choi

ABSTRACT

Incomplete mixing models have recently been shown to better represent solute transport at junctions of pressurized water systems, compared to a complete mixing assumption. The present work incorporated an incomplete solute mixing model into a methodology for sensor network design. Water quality simulations conducted using both mixing models were carried out to generate pollution matrices that provided the input data for the set covering optimization formulation. Multiple contamination and detection scenarios were simulated by considering both a minimum hazard level of the contaminant and a maximum volume of contaminated water consumed. Examination and comparison of outcomes demonstrated that the water quality solver used may impact sensor network designs in three ways by altering: (i) the minimum number of monitoring stations required for full detection coverage, (ii) the optimal layout of stations over the water network and (iii) the detection domain of some stations.

Key words | contamination, optimization, sensor, solute mixing, water system

Pedro Romero-Gomez
Christopher Y. Choi (corresponding author)
Department of Agricultural and
Biosystems Engineering,
University of Arizona,
Tucson,
AZ 85721-0038,
USA
E-mail: cchoi@arizona.edu

Kevin E. Lansey
Department of Civil Engineering and
Engineering Mechanics,
University of Arizona,
PO Box 210072,
Tucson,
AZ 85721,
USA

INTRODUCTION

Early warning monitoring systems deployed in municipal water networks seek to protect the public health and to reduce the effects of contamination incidents. Because each water system is unique, a persistent problem is locating water quality monitoring stations in such a way that they will provide large spatial detection coverage, detect contaminant intrusions promptly and minimize the system's vulnerability. Potential health effects have spurred research on the technical guidelines for designing sensor networks, i.e. finding a set of locations that can provide, with the highest possible certainty, information on the water quality status over the entire piping system.

The starting point of most Sensor Network Design (SND) methodologies is the definition of contamination scenarios characterized by timing and location of pollutant intrusions. Their resulting impacts are evaluated by hydraulic and water quality (WQ) simulations. Optimal sensor locations can then be determined to detect these intrusions based on one or more objective functions. Core assumptions in these meth-

odologies are that the scenarios are representative of possible events and that the simulation models are representative of the system. Here, we examine the impact of an incomplete mixing model on SNDs.

Approaches to SND have evolved along with the capabilities of the computational solvers used to predict hydraulic variables (e.g. water demand, pressure, tank level, etc.) and water quality variables (e.g. water age, tracer, chlorine concentration, etc.). In an early SND study, [Lee & Deininger \(1992\)](#) analyzed the flow pathways using hydraulic simulations and maximized the percentage of the total water demand that was actually covered or monitored by a set of stations using integer programming. In a subsequent study, [Kumar *et al.* \(1997\)](#) eliminated the need for integer programming by performing hydraulic simulations that produced a means of ranking nodes according to the demand covered and selected the locations with highest rank as monitoring stations. They also demonstrated that the optimal sensor location lay on the node with the lowest water quality

(or with the lowest chlorine concentration as predicted by simulations), thus recognizing the role of water quality behavior in SND.

Kessler *et al.* (1998) fully employed the results of their water quality simulations as the primary input data for optimal placement of monitoring stations capable of detecting accidental contaminations in water networks. Their optimal locations were a function of the volume of contaminated water consumed prior to detection, defined as the Level of Service (LOS). A small LOS prevented the pollutant from further propagating to other zones of the network at the expense of more monitoring stations. A binary pollution matrix summarized the contamination status for each contemplated scenario and was the basis for formulating a set-covering optimization problem. In a similar fashion, Ostfeld *et al.* (2004) implemented a Minimum Hazard Level (MHL) within the optimization procedure in order to examine the impact of dilution in the water network. Additionally, the sensing capacity of monitoring stations (Minimum Detection Level, MDL) was taken into account as SND parameters.

Research efforts subsequent to these have dealt with solving multi-objective formulations, reducing the computational time required when the method was applied to real-world water networks, and more realistically considering water utility needs. Ostfeld *et al.* (2008) summarized the state-of-the-art methodologies used in solving the problem presented by various researchers in the Battle of Water Sensor Networks (BWSN). Despite the latest advances in SND methodologies, a research gap remains with respect to the integration and evaluation of more accurate water quality solvers. Using a WQ model that assumed incomplete mixing at cross junctions, Romero-Gomez *et al.* (2008a) produced steady-state-based designs. They found that making such a mixing assumption had significant effects on the layout of sensors over a small network as well as over a highly-interconnected piping system with multiple cross junctions.

The present study thoroughly examines the impact of water quality solvers on sensor network designs intended for networks with multiple types of four-way junctions subject to transient hydraulics. In the present study, in an effort to evaluate alternative contamination events under the incomplete mixing assumption, we utilized the formulation developed by Ostfeld *et al.* (2004) in conjunction with the

improved WQ model, AZRED. These outcomes are then compared against designs based on the conventional complete mixing assumption.

INCOMPLETE SOLUTE MIXING AT JUNCTIONS

Water quality solvers are mathematical models that describe the underlying transport mechanisms of constituents in piping networks. These solvers have been developed on the basis of the convenient but potentially erroneous assumption that solute mixing is complete and instantaneous at the pipe junctions. Figure 1(a) depicts a typical cross junction in order to explain solute transport at a cross junction involving two adjacent inflows and two outflows. The tracer concentrations in both outlet pipes would be equal under the complete mixing assumption. As Fowler & Jones (1991) first pointed out, completely mixed conditions can differ greatly from measurements obtained directly during laboratory and field observations. By making preliminary computational and experimental comparisons of impinging flows with different qualities, van Bloemen Waanders *et al.* (2005) found that the assumption of complete mixing failed to represent the mass transport at cross junctions. Romero-Gomez *et al.* (2006, 2008b), Ho *et al.* (2006) and Austin *et al.* (2008) examined tracer mixing at cross and double-T junctions involving two adjacent inflows and two outflows; all came to the general conclusion that the expected complete mixing does not take place under most flow configurations. Instead, the limited spatio-temporal interaction that occurs between the two inflows actually produces a rather incomplete tracer mixing, with most of the incoming tracer being deflected to the adjacent outlet and only a small amount crossing the junction to the opposite outlet.

In an effort to improve on solvers that rely on the perfect mixing assumption, researchers at the University of Arizona recently incorporated incomplete mixing at junctions into the WQ model. Named AZRED (“AZ” refers to the University of Arizona and “RED” means “network” in Spanish), this solver integrated experimental datasets of tracer mixing at single cross, double-T and Y junctions (as shown in Figure 1) under multiple flow configurations.

The experimental databases summarized the level of mixing as a function of (i) the ratio of incoming flow rates, (ii) the

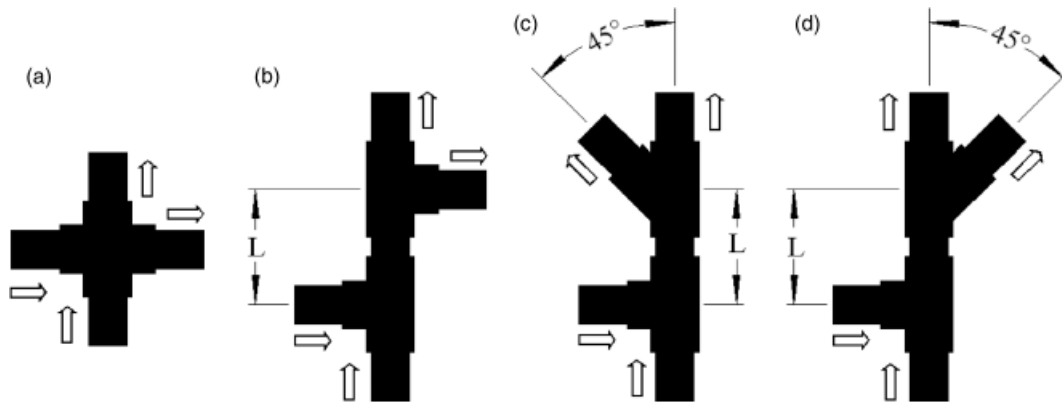


Figure 1 | Junctions and flow configurations at which AZRED applies incomplete mixing of constituents. (a) cross (X), (b) double-T (N), (c) T and Y junctions in U-shape (YU) and (d) T and Y junctions in N-shape (YN). Length (L) indicates the distance between junctions.

ratio of outgoing flow rates and (iii) the type of junction. These databases, along with functions designed to retrieve and to process the data, were incorporated into the EPANET C/C++ code. EPANET performs extended period simulations of the hydraulics and water quality of pressurized water systems (Rossman 2000). Thus, AZRED takes advantage of most of EPANET's built-in functions in order to implement incomplete mixing at the junction types depicted in Figure 1. The performance of AZRED on a laboratory-scale water network has been experimentally examined and validated (Song *et al.* 2009) and applied to a large-scale network (Choi *et al.* 2008). The present study uses AZRED as the WQ modeling tool for providing the input data needed to carry out sensor network designs over two exemplary water networks.

PROCEDURE

Pollution matrix

As the first step of the methodology adopted from Ostfeld *et al.* (2004), a pollution matrix is constructed. This pollution matrix has binary elements in which the i th row lists all nodes contaminated when an intrusion at node i occurs and, consequently, its j th column lists all pollution nodes that contaminate the j th node. Two parameters are defined to establish the criteria for determining whether a node becomes contaminated or not. First, a node is considered contaminated when its concentration becomes greater than a Minimum Hazard Level (MHL). Second, it is assumed that a contaminant continues to propagate throughout the water

network until a volume of contaminated water (the Level of Exposure, LOE) is consumed.

Once these two parameters have been established, then the pollution matrix can be constructed in the following manner. Here, we assume that each network node will be impacted individually at the simulation's starting time. This procedure is general and allows for any contamination location and timing.

1. Set all the elements (λ_{ij}) of the pollution matrix equal to zero.
2. Introduce a contaminant intrusion at the i th node of the water network at time zero.
3. Carry out an Extended-Period WQ Simulation (EPS) using EPANET or AZRED. At each quality time step, calculate the cumulative volume of contaminated water consumed (withdrawn from the network) at nodes in which the concentration is greater than the MHL. Enter $\lambda_{ij}=1$ if the concentration at the j th node is greater than the MHL.
4. If the total contaminated water volume consumed is equal to or greater than LOE, stop the simulation; otherwise, continue until the end of the simulation.
5. Reinitialize the network and return to Step 2 to set the intrusion at the $(i+1)$ -th node.
6. Repeat Steps 2–5 for the N nodes considered to be potential intrusion locations.

Mathematical formulation

After the pollution matrix is constructed, the next step is to find the minimum number of sensors that detect a prescribed proportion of all of the intrusions within the water network

(100% for the examples in this study, full coverage). This problem type, known as a Set Covering Problem (SCP), has wide applications and has been the subject of numerous studies (e.g. Beasley 1987; Beasley & Jørnsten 1992). The SND formulation seeks to find the minimal set of columns (sensor locations) that include at least one value of 1 in all the rows of the pollution matrix (i.e. nodes impacted by each intrusion). The decision variable takes the form of a binary vector (X_j) that indicates all sensor locations. If a sensor should be installed at node j , the j th element of vector X is equal to 1; otherwise X_j is equal to 0. Mathematically, the problem can be defined as follows:

$$\text{Minimize } \sum_{j=1}^N C_j X_j$$

$$\text{subject to } \sum_{j=1}^N \lambda_{ij} X_j \geq 1 \quad i = 1 \dots N$$

$$X_j \in \{0, 1\}, \quad j = 1 \dots N$$

where $\lambda_{i,j}$ is the binary matrix (i,j) term. According to Ostfeld et al. (2004), in order to maximize the overlapping of sensor coverage (so that a contaminant intrusion may be detected by more than one sensor, if possible), a cost C_j can be assigned to each column by using the following formula:

$$C_j = G - \sum_{i=1}^N \lambda_{i,j} \quad \text{where } G \gg N.$$

This minimization problem is solved by using the software Premium Solver for Excel (Frontline Systems, Inc., Incline Village, NV). The solution procedure is carried out on a user-friendly spreadsheet interface on which solution settings are selected (constraints, decision variables, variable type, etc.). Given that the SCP problem is non-convex and NP-hard, a global minimum is not guaranteed.

Modeling modifications to account for incomplete mixing

Conventionally, junctions in hydraulic models are represented as single nodes at which links of different diameter may connect. However, to account for incomplete mixing, an AZRED user should modify the configuration of four-way

junctions. First, the base demand at the junction node is set to zero. Next, four nodes, all at the same elevation and also having no water demand, are added around the junction. These dummy nodes connect to the junction node through short links (relative to the overall network size) with diameters that fall between the smallest and largest diameters of the links that are physically connected to the junction. These changes are consistent with the physical conditions under which the incomplete mixing experiments were conducted. In AZRED's Graphical User Interface (or GUI), the added pipes are drawn to resemble the junction type that they represent, i.e. an N-type arrangement (as shown in Figure 1(b)) should follow a straight line from which two nearly perpendicular links branch out in opposite directions. The combination of angles formed by the links directs AZRED to search for the level of mixing in the correct database according to the junction type.

ILLUSTRATIVE EXAMPLES

A simple network configuration

Net1 of the EPANET examples consists of 12 pipes, 9 nodes (only 8 with base demands), a water source, a pump and an elevated tank. The base demands are subject to a transient pattern throughout a simulation period of 24 h. The remaining characteristics are readily accessible for downloading from the EPANET software package. A few of the original network's features were modified. The initial quality at all nodes was set equal to zero so that only the injected pollutant had an effect on nodal concentrations. Node 22 was modified to conform to the AZRED requirements for four-way junctions. Since the remaining junctions connected two or three pipes, they were not changed. Eight nodes are defined as possible contaminant sources. The resulting network configuration is shown in Figure 2.

Pollution matrices were generated for MHL values of (in mg L^{-1}): 0.5, 1, 1.5, 2, 3 and 4 and LOE values (in m^3) equal to 100, 200, 500, 1000 and 2000. Thus, pollution matrices were prepared and corresponding optimal SNDs were determined for 30 MHL–LOE combinations (six MHL values by five LOE values). Each intrusion was represented by a constant injection mass rate of $10\,000 \text{ mg min}^{-1}$ over the entire

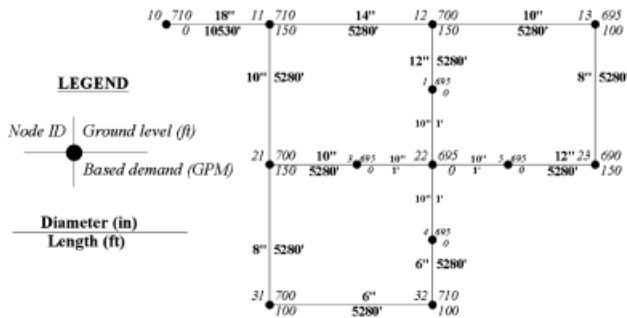


Figure 2 | Example network 1 (modified Net1.net from EPANET examples).

simulation period, starting from time zero for all intrusion scenarios.

Comparing EPANET and AZRED results showed that the designs differed following one of three cases: (i) the minimum number of sensors is different, (ii) the minimum number of sensors is the same but some locations are different and (iii) the minimum number of sensors and the optimal locations are the same but some sensors have different detection domains.

Of the 30 designs that were obtained for Net1, five (indicated as MHL-LOE combinations A, B, C, D and F in Figure 3) required a different number of sensors owing to the WQ solver used, whereas one design (combination E) required the same number of sensors but they were placed at different locations. In the former five results, EPANET-based designs required more sensors than AZRED-based

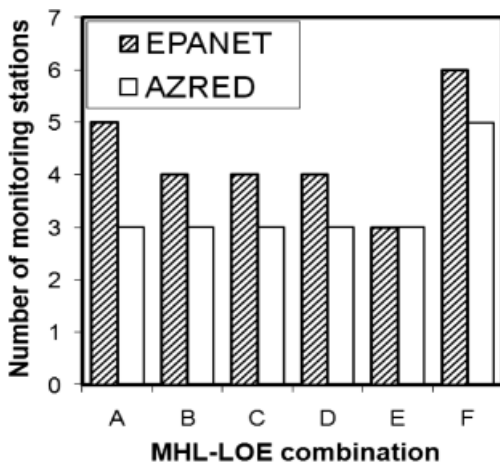


Figure 3 | Number of stations based on EPANET and AZRED at different MHL-LOE combinations. (A) MHL = 1.5 mg L⁻¹, LOE = 1000 m³, (B) MHL = 1.5 mg L⁻¹, LOE = 2000 m³, (C) MHL = 2 mg L⁻¹, LOE = 500 m³, (D) MHL = 2 mg L⁻¹, LOE = 1000 m³, (E) MHL = 2 mg L⁻¹, LOE = 2000 m³, (F) MHL = 3 mg L⁻¹, LOE = 100 m³.

designs, with the greatest difference (two stations) resulting from combination A (MHL = 1.5 mg L⁻¹, LOE = 1000 m³). Such a difference was deemed significantly large with respect to the maximum number of sensors possible for this network (eight possible stations). Under incomplete mixing, the presence of the cross junction at node 22 produced a tendency for the contaminant to be concentrated at one of the two downstream nodes (nodes 23 and 32). Greater levels of mixing (as produced by EPANET) will cause the contaminant to spread more widely at lower concentrations, making detection more difficult and requiring more monitoring stations.

This network is simple enough to allow for a detailed observation of the contaminant pathways. For example, when contamination is introduced at node 21 (Figure 2), the complete mixing outcome was that clear (or non-contaminated) water from node 12 reduced the contaminant concentration in the pipes leaving node 22, dispersing the contaminant but at lower concentrations. In contrast, in the incomplete mixing analysis in AZRED most of the contaminant entering node 22 was deflected towards node 32 with little mixing. The transient concentration patterns at node 32, as obtained using both WQ solvers, are shown in Figure 4. Node 32's AZRED concentration exceeds the MHL value of 2 mg L⁻¹ established for combination C while the complete mixing model does not. Consequently, this difference was reflected in the pollution matrices generated by both WQ

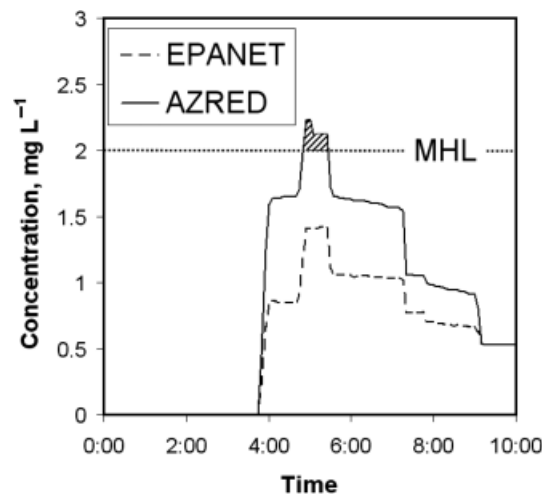


Figure 4 | EPS contaminant concentration at node 32, based on EPANET and AZRED solvers. Shaded area indicates concentrations higher than 2 mg L⁻¹ (dotted horizontal line, MHL setting for combination C).

Table 1 | Pollution matrix resulting from MHL–LOE combination C, based on (a) EPANET and (b) AZRED

(a)	Injection node	Contaminated node							
		10	11	12	13	21	23	31	32
	10	0	0	0	0	0	0	0	0
	11	0	1	0	0	1	0	1	0
	12	0	1	1	1	0	0	0	0
	13	0	0	0	1	0	1	0	0
	21	0	0	0	0	1	0	1	0
	23	0	0	0	0	0	1	0	0
	31	0	0	0	0	0	0	1	1
	32	0	0	0	0	0	0	0	1

(b)	Injection node	Contaminated node							
		10	11	12	13	21	23	31	32
	10	0	0	0	0	0	0	0	0
	11	0	1	0	0	1	0	1	0
	12	0	1	1	1	0	0	0	0
	13	0	0	0	1	0	1	0	0
	21	0	0	0	0	1	0	1	1
	23	0	0	0	0	0	1	0	0
	31	0	0	0	0	0	0	1	1
	32	0	0	0	0	0	0	0	1

solvers, as indicated in Table 1. A value of 0 is set in the source row for node 21 and column (detection location) for node 32 in Table 1(a). On the other hand, the same matrix element in Table 1(b), generated by AZRED, is set to 1. The other elements remained the same regardless of the WQ solver. Figure 4 also shows that AZRED will not influence the time that a contaminant arrives at a potential sensor location; it will only influence the transient concentrations. Therefore, we do not expect AZRED to have any impact on the formulations that account for detection time as one of the objective functions (Cozzolino et al. 2006; Preis & Ostfeld 2008). Also, we do not expect sensor network designs will require any changes when the MHL value is equal to zero.

Despite the minor difference between the two pollution matrices, the optimal SNDs were noticeably different (Figures 5(a, b)). Based on the EPANET WQ predictions, a minimum of four stations is required in order to detect intrusions from any of the eight source nodes (Figure 5(a)). On the other hand, the AZRED-based design for this combi-

nation of parameters (combination C, Figure 3) requires only three monitoring stations in order to achieve full detection coverage (Figure 5(b)). In a broader context, such a difference directly increases the cost of the network. Detection domains in Figure 5 show which nodes are covered by each station; for instance, injections at nodes 11 and 12 will be detected by the station placed at node 11 for both EPANET and AZRED-based designs.

Figure 5(c, d) depict conditions (combination E: MHL = 2 mg L⁻¹, LOE = 2000 m³) under which the number of required stations remained the same but their locations were different due to the mixing assumption. To provide full coverage under this combination, the monitoring station that was placed at node 11 using the EPANET results was shifted to node 21 based on AZRED outcomes. In addition to providing full coverage, a monitoring station at node 21 increases sensor overlap as compared to the optimal design that included a sensor at node 11. A monitoring system's ability to detect contaminant intrusions at more than one station is particularly desirable when the probability of sensor failure is taken into consideration as part of the SND methodology.

Figure 5(e, f) present the EPANET- and AZRED-based designs, respectively, for MHL = 1 mg L⁻¹ and LOE = 500 m³, in which the number and location of stations are the same but the detection domains are different. Although the EPANET-based design maximized sensor overlap (Figure 5(e)), injections at node 21 undergo incomplete mixing at the cross that prevents the contaminant concentrations at node 23 from reaching the MHL. Consequently, the detection domain of a station at node 23 is reduced (Figure 5(f)).

Net3 from EPANET examples

Net3 is available with the EPANET download and consists of 117 links, 92 nodes, 3 elevated tanks and 2 pumps that connect to 2 reservoirs (a lake and a river). Eight nodes from the network are four-way junctions (Table 2) and were modified to each of the four alternative AZRED junction types (Figure 1). The location and characteristics of the modified junctions are listed in Table 2. Contamination scenarios were developed with each of the 84 nodes being equally likely as the contamination source. The quality time step and parameter were set at 2 min and "Chemical",

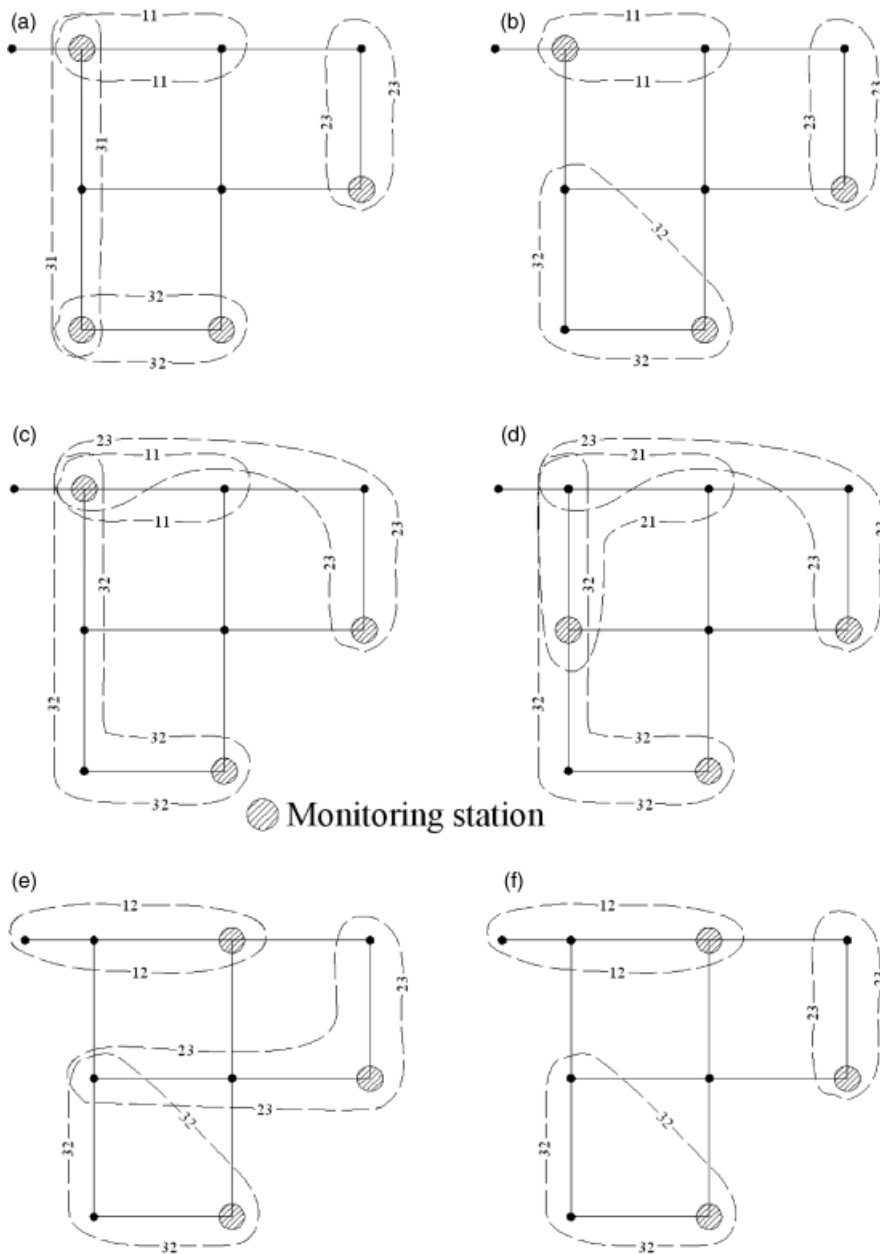


Figure 5 | Sensor Network Designs based on (a) EPANET and (b) AZRED for combination C ($MHL=2 \text{ mg L}^{-1}$, $LOE=500 \text{ m}^3$); (c) EPANET and (d) AZRED for combination E ($MHL=2 \text{ mg L}^{-1}$, $LOE=2000 \text{ m}^3$) and (e) EPANET and (f) AZRED for combination $MHL=1 \text{ mg L}^{-1}$ and $LOE=500 \text{ m}^3$. Dashed lines indicate the coverage domain of each monitoring station.

respectively. The injection mass rate was set equal to $20000 \text{ mg min}^{-1}$ for all the scenarios. The settings for the MHL value (in mg L^{-1}) were 1, 2, 5, 10, 20, 50 and 100, and the LOE settings (in m^3) were 50, 100, 200, 500, 1000, 2000 and 5000. Pollution matrices were thus developed for all 49 MHL-LOE combinations.

The tradeoff between the level of exposure and the number of monitoring stations for $MHL=1 \text{ mg L}^{-1}$ is presented in

Figure 6(a). The LOE level indirectly implies the time till a contamination event is detected. Higher LOE values allow for more consumption (or usage) of contaminated water and, consequently, longer times to detection and further pollutant propagation. Thus, fewer stations are needed and, as seen in Figure 6(a), the number of stations decreases monotonically with increasing LOE. This decreasing trend is consistent with Ostfeld *et al.* (2004) and occurred for all sensor arrangements

Table 2 | Modifications on Net3 example from EPANET package

Node ID	Type	Junction diameter (<i>D</i>)	Dimensionless distance (<i>L/D</i>)
121	YN	24	2.0
120	N	10	1.0
119	X	20	N/A
115	YN	10	3.0
193	N	12	2.5
189	X	12	N/A
255	YU	10	1.0
111	YN	12	2.0

regardless of the mixing model applied. However, AZRED simulations generally required one more station for most LOE values. Therefore, if complete mixing at junctions is applied, some nodes will not be covered by any of the sensors. This monitoring deficiency could become critical if the potential entry points were assigned a likelihood of contamination.

Figure 6(b) illustrates the tradeoff between the Minimum Hazard Level (MHL) and the number of monitoring stations for $LOE = 100 \text{ m}^3$. Lower MHLs will result with more sensitive sensors that can detect at lower concentrations. Thus, more dilute contaminations could be identified and fewer stations are likely to be necessary for the same protection. At the other end of the spectrum ($MHL > 20 \text{ mg L}^{-1}$ in this exemplary case), few nodes become contaminated and monitoring stations are only needed at those locations.

Comparing SNDs for the alternative WQ modeling assumptions for the 49 MHL–LOE combinations, differences in designs occurred primarily for conservative parameter sets, i.e. when detection capacity was enhanced ($MHL \leq 10 \text{ mg L}^{-1}$) or when the propagation of polluted water was restricted ($LOE \leq 200 \text{ m}^3$) (Table 3). No specific trend was identified in relation to the minimum number of sensors needed for either WQ solver.

To understand the spatial differences in SND, the frequencies that a node was selected by both water-quality-model-based SNDs were computed and compared for the 49 MHL–LOE combinations. It was found that nodes with the highest frequency corresponded to the locations that were more sensitive to the solute mixing model used to generate the pollution matrices. Figure 7 shows that sensitive nodes generally lie near four-way junctions, a finding that suggests the effect will be stronger at locations in close proximity to a junction and weaker further downstream from four-way junctions. Circle sizes in Figure 7 represent the magnitude of the difference in frequencies; the small circles indicate the node was selected one more time by one of the models and the larger circles are differences greater than one. This analysis also reveals that certain patterns will occur according to the water quality model used. For instance, nodes 211 and 213 were consistently interchangeable (as were nodes 247 and 251) due to the influence of a YU-type junction (node 255) and the elevated tank (tank 2) that produced highly dynamic hydraulics in this region.

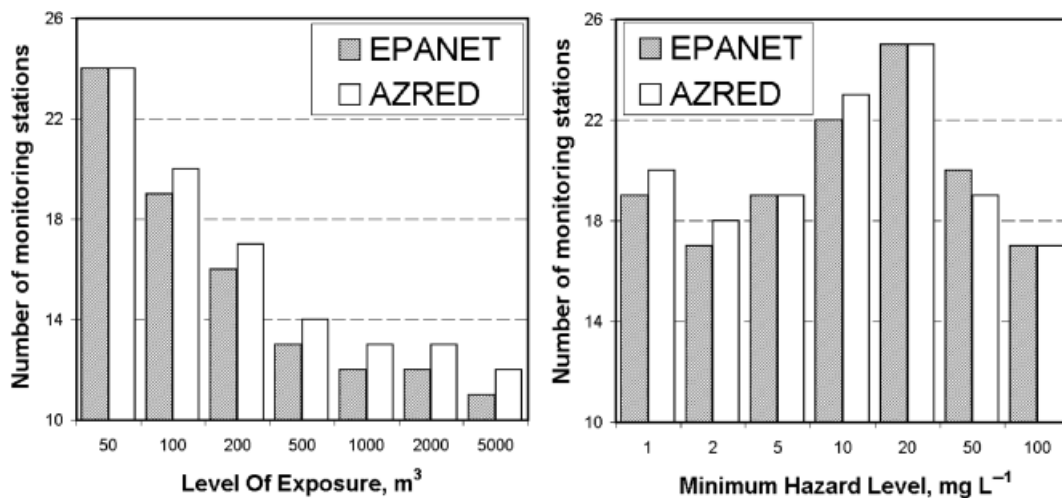
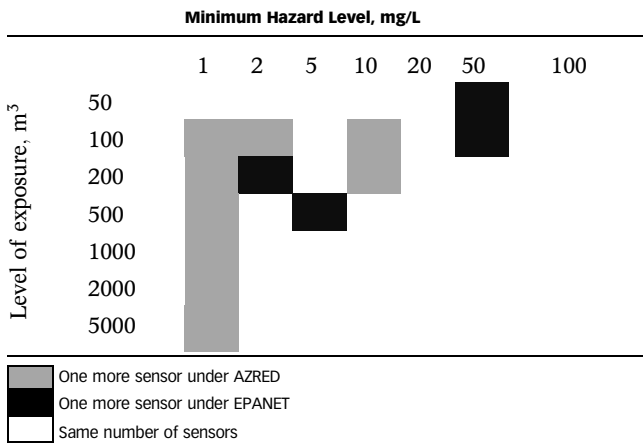
**Figure 6** | Number of monitoring stations as a function of (a) the Level Of Exposure (LOE) for $MHL = 1 \text{ mg L}^{-1}$ and (b) the Minimum Hazard Level (MHL) for $LOE = 100 \text{ m}^3$.

Table 3 | Summary of number of sensor based on AZRED vs. EPANET simulations

CONCLUSIONS

This study incorporated a newly developed incomplete solute mixing model in an optimal sensor network design. We analyzed various contamination and detection scenarios over two exemplary networks. The first example system allowed for detailed descriptions of the contaminant pathway and its ultimate impact on each design while the second exemplary network provided the global trends of the impact on the sensor configurations due to the WQ solvers. Application results demonstrated that, while the quantitative impact of incomplete mixing varies, EPANET-based designs did not

provide the desired level of protection under incomplete mixing conditions. For example, several AZRED-based designs allocated more monitoring stations than EPANET-based design; this strongly implies that an EPANET-based sensor arrangement will fail to detect some contaminant intrusions.

Overall, these results demonstrate that water quality modeling tools and their accuracy play a central role in SND. Developing and implementing SND approaches should necessarily entail a continuous re-examination in view of the new advances in water quality modeling in pressurized piping systems. More accurate solute transport predictions will make SNDs account for a better representation of the water quality behavior of a network. This in turn should increase the effectiveness and reliability of early warning monitoring systems as well as the likelihood that intentional or accidental contamination events can be detected early enough to initiate countermeasures that can protect water customers.

ACKNOWLEDGEMENTS

This work is supported by the Environmental Protection Agency/Department of Homeland Security (under Grant No. 613383D).

REFERENCES

- Austin, R. G., van Bloemen Waanders, B., McKenna, S. & Choi, C. Y. 2008 *Mixing at cross junctions in water distribution systems. II: Experimental study*. *J. Wat. Res. Plann. Mngmnt.* **134** (3), 295–302.
- Beasley, J. E. 1987 *An algorithm for set covering problem*. *Eur. J. Oper. Res.* **31**, 85–95.
- Beasley, J. E. & Jørnsten, K. 1992 *Enhancing an algorithm for set covering problems*. *Eur. J. Oper. Res.* **58**, 293–300.
- Choi, C. Y., Shen, J. Y. & Austin, R. G. 2008 *Development of a comprehensive solute mixing model (AZRED) for double-tee, cross, and wye junctions*. In: *Proc. 10th Annual Water Distribution Systems Analysis Symposium*. ASCE Library, Kruger National Park, South Africa, pp. 1004–1014. Also, note the website <http://cals.arizona.edu/~cchoi/AZRED>
- Cozzolino, L., Mucherino, C., Pianese, D. & Pirozzi, F. 2006 *Positioning, within water distribution networks, of monitoring stations aiming at an early detection of intentional contamination*. *Civil Engng. Environ. Syst.* **23**(3), 161–174.
- Fowler, A. G. & Jones, P. 1991 *Simulation of water quality in water distribution systems*. In: *Proc. Water Quality Modeling in Distribution Systems*. AWWARF/EPA, Cincinnati, OH.

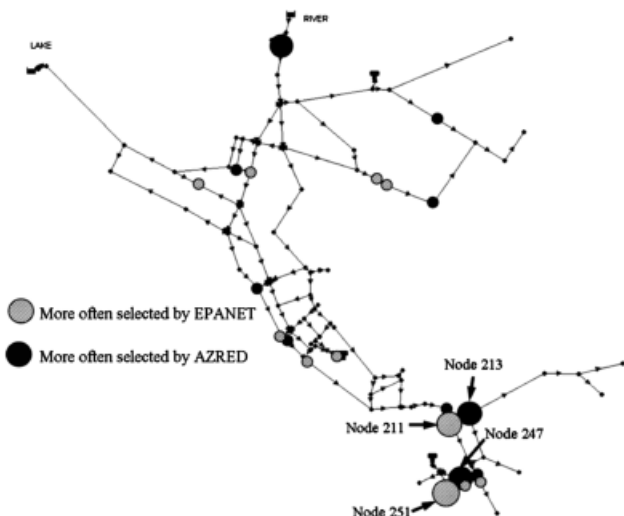


Figure 7 | Nodes that were more often selected as optimal sensor locations based on EPANET or on AZRED WQ simulations. Circle sizes indicate higher sensitivity to the solute mixing model.

- Ho, C. K., Orear, L., Wright, J. L. & McKenna, S. A. 2006 Contaminant mixing at pipe joints: comparison between laboratory flow experiments and computational fluid dynamics models. In: *Proc., 8th Annual Water Distribution System Analysis Symp., Cincinnati, OH*. ASCE Library, Cincinnati, OH.
- Kessler, A., Ostfeld, A. & Sinai, G. 1998 **Detecting accidental contaminations in municipal water networks**. *J. Wat. Res. Plann. Mngmnt.* **124**(4), 192–198.
- Kumar, A., Kansal, M. L. & Arora, G. 1997 **Identification of monitoring stations in water distribution system**. *J. Environ. Engng.* **123**(8), 746–752.
- Lee, B. H. & Deininger R. A. 1992 **Optimal locations of monitoring stations in water distribution system**. *J. Environ. Engng.* **118**(1), 4–16.
- Ostfeld, A., Kessler, A. & Goldberg, I. 2004 **A contaminant detection system for early warning in water distribution networks**. *Engng. Optim.* **36**(5), 525–538.
- Ostfeld, A., Uber, J. G., Salomons, E., Berry, J. W., Hart, W. E., Phillips, C. A., Watson, J.-P., Dorini, G., Jonkergouw, P., Kapelan, Z., di Pierro, F., Khu, S.-T., Savic, D., Eliades, D., Polycarpou, M., Ghimire, S. R., Barkdoll, B. D., Gueli, R., Huang, J. J., McBean, E. A., James, W., Krause, A., Leskovec, J., Isovitsch, S., Xu, J., Guestrin, C., VanBriesen, J., Small, M., Fischbeck, P., Preis, A., Propato, M., Piller, O., Trachtman, G. B., Wu, Z. Y. & Walski, T. 2008 **The battle of the water sensor networks (BWSN): a design challenge for engineers and algorithms**. *J. Wat. Res. Plann. Mngmnt.* **134**(6), 556–568.
- Preis, A. & Ostfeld, A. 2008 **Multiobjective contaminant sensor network design for water distribution systems**. *J. Wat. Res. Plann. Mngmnt.* **134**(4), 366–377.
- Romero-Gomez, P., Choi, C. Y., Lansey, K. E., Preis, A. & Ostfeld, A. 2008a **Sensor network design with improved water quality models at cross junctions**. In: *Proc. 10th Annual Water Distribution Systems Analysis Symposium*. ASCE Library, Kruger National Park, South Africa, pp. 1056–1064.
- Romero-Gomez, P., Choi, C. Y., van Bloemen Waanders, B. & McKenna, S. A. 2006 **Transport phenomena at intersections of pressurized pipe systems**. In: *Proc. 8th Annual Water Distribution System Analysis Symp., Cincinnati, OH*. ASCE Library, Cincinnati, OH.
- Romero-Gomez, P., Ho, C. K. & Choi, C. Y. 2008b **Mixing at cross junctions in water distribution systems. I: Numerical study**. *J. Wat. Res. Plann. Mngmnt.* **134**(3), 285–294.
- Rossman, L. 2000 *EPANET User's Manual*. USEPA, Cincinnati, OH.
- Song, I., Romero-Gomez, P. & Choi, C. Y. 2009 **Experimental verification of incomplete solute mixing in a pressurized pipe network with multiple cross junctions**. *J. Hydraul. Engng.* **135**(11), 1005–1011.
- van Bloemen Waanders, B., Hammond, G., Shadid, J., Collis, S. & Murray, R. 2005 **A comparison of Navier–Stokes and network models to predict chemical transport in municipal water distribution systems**. In: *Proc., World Water and Environmental Resources Congress, Anchorage, AK*. ASCE Library, Anchorage, AK.

First received 25 January 2010; accepted in revised form 5 June 2010. Available online 9 November 2010