

## Grain-size model for hydraulic conductivity estimation of porous media

Abhishish Chandel \* and Vijay Shankar 

National Institute of Technology, Hamirpur, India

\*Corresponding author. E-mail: abhishishchandel@gmail.com

 AC, 0000-0002-3082-2721; VS, 0000-0002-9509-6804

### ABSTRACT

Hydraulic conductivity ( $k$ ) is an important hydraulic parameter of porous media, and its accurate prediction plays a vital role in sub-surface flow investigations. The study uses the borehole soil samples to develop a  $k$  model using parameters, effective grain-size ( $d_{10}$ ), and standard deviation ( $\sigma$ ). The influence of  $d_{10}$  and  $\sigma$  on the  $k$  and evaluation of  $k$  values via four empirical models is also assessed in this study. For soil samples, the  $k$  increases with the increase in the  $d_{10}$  grain-size and decreases with the increase in the  $\sigma$  value. The evaluation of  $k$  via empirical models infers that the Hazen model performs well in the estimation of  $k$  values. The evaluation of  $k$  using various statistical indicators points towards low error statistics (MAE, RMSE, and BIAS) and high determination coefficient ( $R^2$ ) between the measured and developed model-based  $k$  values, which substantiate the efficacy of the developed model as compared to the existing empirical models for estimating the  $k$ . To establish the versatility of the developed grain-size model, its validation was done using independent soil samples for estimating  $k$  values.

**Key words:** aquifer, groundwater, hydraulic conductivity, porous media

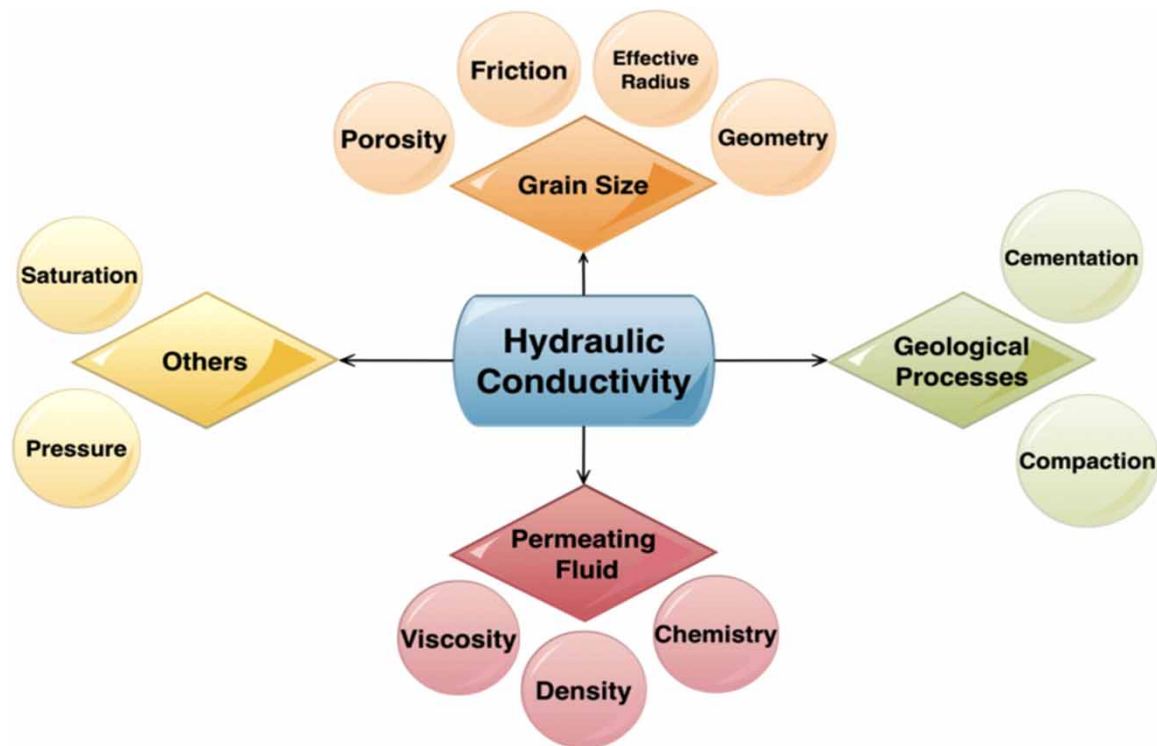
### HIGHLIGHTS

- The research proposes a grain-size model for estimating the hydraulic conductivity of porous media by examining the effect of the  $\sigma/d_{10}$  parameter on  $k$ .
- The established hydraulic conductivity model served as a valuable tool for calculating aquifer yield and groundwater recharge with precise accuracy.

---

This is an Open Access article distributed under the terms of the Creative Commons Attribution Licence (CC BY 4.0), which permits copying, adaptation and redistribution, provided the original work is properly cited (<http://creativecommons.org/licenses/by/4.0/>).

## GRAPHICAL ABSTRACT



## INTRODUCTION

The hydraulic conductivity ( $k$ ) of porous media and its accurate assessment is important for a hydrogeologist in aquifer and groundwater studies (Leroueil *et al.* 2002). Initially, Henry Darcy explained the concept of hydraulic conductivity and defined it as the ease with which the flow of fluid takes place through the interconnected pore space (Pucko & Verbovsek 2015; Chandel *et al.* 2022a). For porous media, the studies concerning the  $k$  estimation are significant to govern the percolation ability of the permeable bed and soil sediments, as these factors are reliant on groundwater recharge (Chandel *et al.* 2022b).

In the previous studies, various researchers namely Parvazinia *et al.* (2006), Kundu *et al.* (2016), and Singh *et al.* (2021) studied different approaches for modelling the  $k$  of flow through porous media. The outcomes of these studies infer some restrictions, i.e., simulating the infinite extent of sub-surface flow. One of the important considerations is to control other factors that influence the  $k$  of porous media while examining the influence of individual parameters, i.e., standard deviation and mean and effective size (Lu *et al.* 2012). Pliakas & Petalas (2011) developed a statistical regression model by examining the correlation between the  $k$  and grain-size parameters and postulated that the developed model performed better in the estimation of  $k$  values. Salarashayeri & Siosemarde (2012) proposed a hydraulic conductivity equation using grain diameters, i.e.,  $d_{10}$ ,  $d_{50}$ , and  $d_{60}$ . The study recommended that the grain diameter ( $d_{10}$ ) is an influencing factor for the  $k$  estimation of porous media. The M5 model tree regression approach was used to develop a  $k$  model based on the grain-size data (Naej *et al.* 2017). Wang *et al.* (2017) proposed a  $k$  model by performing the dimensional analysis between the particle size parameters with the  $k$ . The developed equation performs better in the estimation of  $k$  values as compared to the existing empirical models. Ren & Santamarina (2018) estimated the  $k$  of porous media using the sediment and index characteristics i.e, void ratio, grain-size distribution, and surface area. Toumpanou *et al.* (2021) evaluated the  $k$  of the crushed sand-sized limestone using the pre-existing six empirical equations. Mujtaba *et al.* (2021) correlated the hydraulic conductivity with the different grain-sizes of sandy soil and concluded that the  $d_{10}$  grain-size shows better goodness of fit with the  $k$  as compared to the other grain-sizes. Khaja *et al.* (2022) developed an equation for the estimation of  $k$  of sandy aquifers using the grain-size data. The developed equation gives better results when the grain-size, i.e.,  $d_{30}$  is considered to be an effective parameter.

From the existing literature, it has been seen that limited research work has been done to examine the influence of  $d_{10}$  and  $\sigma$  on the  $k$  of porous media. The standard deviation signifies the non-uniformity of the soil particles, whereas the effective grain-size represents the behavior of the entire grain-size pattern. The study incorporates two parameters, i.e., effective grain-size ( $d_{10}$ ) and standard deviation ( $\sigma$ ) for the development of a grain-size model. The present study is aimed:

- (1) To investigate the influence of  $d_{10}$  and  $\sigma$  on the  $k$  value of porous media.
- (2) To develop a grain-size model using parameters, i.e.,  $\sigma$  &  $d_{10}$  and evaluate its efficacy in estimating the  $k$  of porous media.
- (3) To validate the grain-size model using data set of independent soil samples.

## MATERIALS AND METHODOLOGY

### Materials

The study uses 27 borehole soil samples, which were attained during the ongoing drilling process from the Chamba district of Himachal Pradesh in India. Thin-walled sampler tubes were used to collect the undisturbed soil samples. For experimental work, the soil samples were collected at an interval of 3.5 m from the core material.

### Methodology

The collected soil samples were subjected to the grain-size analysis to compute the mean and effective size. The pycnometer method was used to determine the specific gravity of soil samples, which is helpful in the determination of porosity. The grain-size curve (Figure 2) is used to determine the effective grain-size ( $d_{10}$ ), whereas the standard deviation ( $\sigma$ ) is determined (Rushton 2004) using the equation:

$$\sigma = \left( \sum (d_{50} - d_i)^2 * \Delta Z_i \right)^{\frac{1}{2}} \quad (1)$$

where,  $d_{50}$  – mean grain-size,  $\Delta Z_i$  – total particles fraction, and  $d_i$  – particle size retained on a particular sieve.

Initially, the influence of  $d_{10}$  and  $\sigma$  on the  $k$  of soil samples were examined individually, and then by investigating the behaviour of  $k$  with the different  $\sigma/d_{10}$  values, a grain-size model has been proposed to predict hydraulic conductivity. For hydraulic conductivity estimation of soil samples, a constant head permeameter test has been performed. Permeameters of different diameters (5.08, 10.16, and 15.26 cm) have been used for  $k$  estimation as shown in Figure 1.

Figure 1 indicates the hydraulic conductivity measuring setup, which includes galvanized iron permeameters of different diameters. For each permeameter, the total and test lengths are 1 m and 0.46 m respectively. An overhead tank is provided above the permeameter outlet, which provides a continuous flow of water to the permeameter. The arrangement of pressure tapping points is provided along the permeameter periphery, which helps in measuring the readings of the manometer. The volume of the water is measured using a measuring cylinder for a particular time interval. The standard methodology as described by Chandel & Shankar (2021) has been followed to determine the  $k$  of soil samples. During the  $k$  measurement, the temperature of the water is recorded at the start and end of the analysis using a digital thermometer (ASTM 2006).

### Empirical models for $k$ assessment

Based on the literature review, various empirical models that are used to evaluate the  $k$  of porous media have been used in this study. These empirical models relate the  $k$  with the grain-size, viscosity of fluid, porosity, sorting coefficient, and uniformity coefficient. Table 1 shows the empirical models with their boundary conditions used in the present study to evaluate the  $k$  values. In the empirical models, the  $\nu$  and  $g$  values were taken as 0.885 mm<sup>2</sup>/s and 981 cm/s<sup>2</sup> respectively.

### Statistical performance indicators

In this study, several statistical indicators namely scatter index ( $S_i$ ), BIAS, agreement index ( $I_a$ ), determination coefficient ( $R^2$ ), root mean square error (RMSE), and mean absolute error (MAE) have been used for quantitative

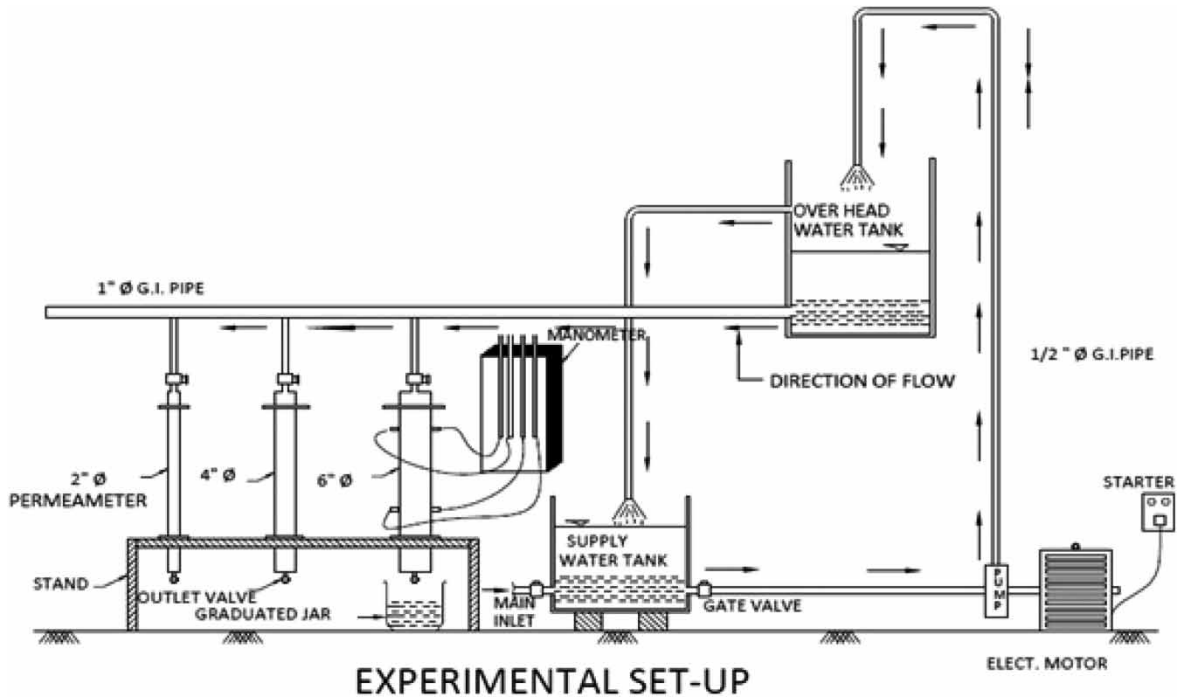


Figure 1 | Hydraulic conductivity measuring setup.

Table 1 | Empirical models for k assessment

Investigator	Equation	Boundary conditions
Hazen (1892)	$k = \frac{g}{v} * 6 \times 10^{-4} * [1 + 10(n - 0.26)] * d_{10}^2$	$d_{10}$ (0.1–3 mm) $U < 5$
Kozeny (1927), Carman (1937, 1956)	$k = \frac{g}{v} * 8.3 \times 10^{-3} * \left[ \frac{n^3}{(1-n)^2} \right] * d_{10}^2$	$d_{10} < 3.0$ mm
Chapuis <i>et al.</i> (2005)	$k = 1.412 * \frac{n^{2.35}}{(1-n)^{1.565}} * d_{10}^{1.565}$	$d_{10}$ (0.03–3 mm)
Naej <i>et al.</i> (2017)	$k = \frac{g}{\vartheta} * 1.84 \times 10^{-4} * d_{10}^{0.85} * U^{-0.55}$	suitable for sand $d_{10} < 3.0$ mm

where, U, uniformity coefficient i.e.,  $d_{60}/d_{10}$ , and n, porosity.

evaluation. The statistical indicators (Chandel *et al.* 2021) are defined as:

$$BIAS = \sum_{i=1}^Z \frac{1}{Z} (C_i - M_i) \tag{2}$$

$$S_i = \frac{\sqrt{\frac{1}{Z} \sum_{i=1}^Z (C_i - M_i)^2}}{M_i} \tag{3}$$

$$R^2 = \left[ \frac{\sum_{i=1}^Z (M_i - \bar{M})(C_i - \bar{C})}{\sqrt{\sum_{i=1}^Z (M_i - \bar{M})^2 \sum_{i=1}^Z (C_i - \bar{C})^2}} \right]^2 \tag{4}$$

$$I_a = 1 - \frac{\sum_{i=1}^Z (C_i - M_i)^2}{\sum_{i=1}^Z |C_i - \bar{M}| + |M_i - \bar{M}|} \tag{5}$$

$$MAE = \frac{1}{Z} \sum_{i=1}^Z |M_i - C_i| \tag{6}$$

$$RMSE = \sqrt{\frac{\sum_{i=1}^Z (M_i - C_i)^2}{Z}} \tag{7}$$

where,  $C_i$  and  $M_i$  represent the computed and measured k values respectively, and  $Z$  is the number of datasets.  $\bar{C}$  and  $\bar{M}$  denote the average values of computed and measured parameters, respectively.

### RESULTS AND DISCUSSION

The hydraulic test and gradation analysis have been conducted on the 27 borehole soil samples. The data points of 15 soil samples were used to develop a k model by examining the influence of  $\sigma/d_{10}$  parameter on the k. The data set of remaining independent soil samples was used for the validation of the developed model.

#### Grain-size analysis

The grain-size analysis was performed to plot the grain-size curve between the particle size and percent finer. Figure 2 represents the grain-size curve for 15 soil samples. The grain sizes corresponding to the 10%, 30%, and 60% finer by weight i.e.,  $d_{10}$ ,  $d_{30}$ , and  $d_{60}$ , standard deviation, and uniformity coefficient values were determined using the grain-size curve. Whereas, the grain-size parameters ( $d_{10}$  and  $\sigma$ ) for the remaining soil samples have been determined and provided in the validation section.

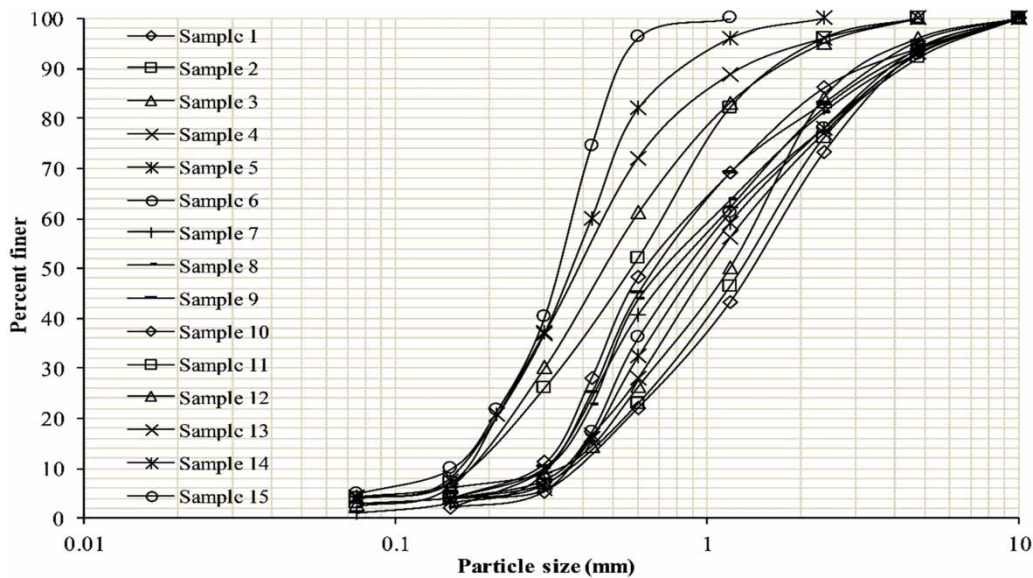


Figure 2 | Grain-size curve of 15 soil samples.

Table 2 represents the basic characteristics of the 15 soil samples. The  $\sigma$  and  $d_{10}$  values vary between 1.470 to 7.100 and 0.173 to 0.386 mm respectively.

#### Variation of hydraulic conductivity with $d_{10}$ and $\sigma$

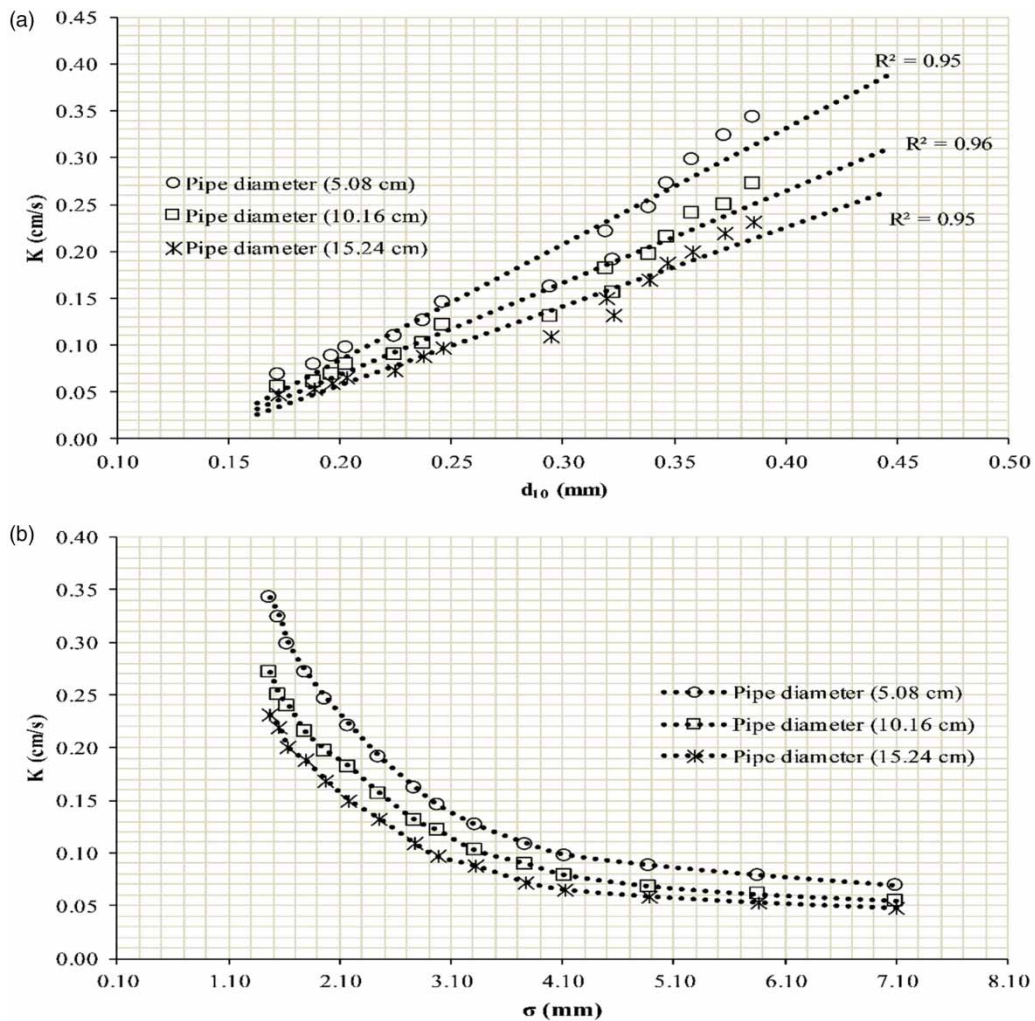
Figure 3(a) represents a linear variation of k values, which are obtained from the three permeameters with the different values of effective grain-size ( $d_{10}$ ). The increase in the  $d_{10}$  grain-size provides more interconnected



**Table 2** | Basic characteristics of the soil samples

Sample no.	$d_{10}$ (mm)	$d_{30}$ (mm)	$d_{50}$ (mm)	$d_{60}$ (mm)	$n^a$	$U^a$	$\sigma^a$
1	0.386	0.821	1.450	1.850	0.359	4.793	1.470
2	0.373	0.774	1.330	1.720	0.363	4.611	1.560
3	0.358	0.690	1.180	1.530	0.370	4.274	1.640
4	0.347	0.631	1.050	1.400	0.375	4.035	1.790
5	0.342	0.575	0.980	1.240	0.385	3.626	1.980
6	0.330	0.544	0.920	1.160	0.387	3.515	2.190
7	0.325	0.499	0.850	1.120	0.389	3.446	2.460
8	0.301	0.486	0.785	1.070	0.386	3.555	2.780
9	0.297	0.468	0.720	0.962	0.394	3.239	2.980
10	0.278	0.442	0.650	0.929	0.392	3.342	3.320
11	0.225	0.346	0.570	0.754	0.392	3.351	3.780
12	0.191	0.300	0.490	0.590	0.398	3.089	4.130
13	0.187	0.264	0.410	0.497	0.410	2.658	4.890
14	0.178	0.260	0.370	0.425	0.418	2.388	5.870
15	0.173	0.252	0.335	0.372	0.426	2.150	7.100

<sup>a</sup>represents the unitless parameters.



**Figure 3** | Hydraulic conductivity variation with (a)  $d_{10}$  (b)  $\sigma$ .

space between the voids for the fluid flow, which results in an increase in the  $k$  value. The outcome of the study is in close agreement with the findings of Cabalar & Akbulut (2016).

Figure 3(b) indicates that the hydraulic conductivity decreases with the increase in the  $\sigma$  values, which vary from 1.47 to 7.10. The  $\sigma$  value more than unity signifies the non-uniformity of soil particles. In this study, the non-uniformity of soil particles increases with the increase in the  $\sigma$  value which results in providing more compactness to the porous particles and thus results in the decreased  $k$  value.

#### Variation of $k$ with $\sigma/d_{10}$

The hydraulic conductivity variation was examined by plotting the  $k$  with different  $\sigma/d_{10}$  values for different diameter permeameters.

The  $\sigma/d_{10}$  parameter effectively incorporates the influence of non-uniformity and gradation characteristics on the  $k$  of porous media. Figure 4 shows that the  $k$  decreases with the increase in the  $\sigma/d_{10}$  value. The curve is concave upward and meets the lower limiting value asymptotically. In Figure 4, the trend of the curve implies that the result of the study is in line with the outcomes of Pliakas & Petalas (2011).

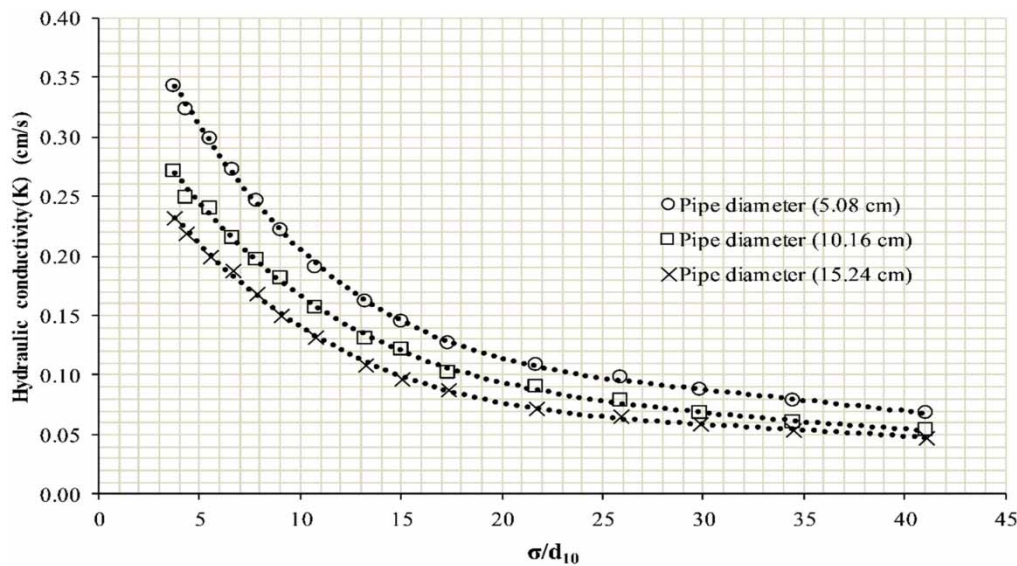


Figure 4 | Variations of  $k$  with  $\sigma/d_{10}$ .

#### Grain-size model development

The data points of 15 soil samples have been used for the development of a grain-size model. For model development, the grain-size parameters ( $\sigma$  and  $d_{10}$ ) have been integrated to form a dimensionless parameter ' $\sigma/d_{10}$ '. The equation of the developed model is derived by investigating the variations of the  $\sigma/d_{10}$  parameter with the hydraulic conductivity values obtained from the three different diameter permeameters for 15 soil samples i.e., a total number of 45  $k$  versus  $\sigma/d_{10}$  variations were examined for model development. The ' $\sigma/d_{10}$ ' parameter in the developed model includes coefficients of degree 0–4. For the development of the model, the principle of least-squares approach is used in this study (Wang *et al.* 2017).

The developed hydraulic conductivity model is:

$$k = \beta * \left[ a_0 + \left\{ \int_{i=1}^{i=4} a_i * \left( \frac{\sigma}{d_{10}} \right)^i \right\} \right] \quad \left( 3.805 \leq \frac{\sigma}{d_{10}} \leq 41.066 \right) \quad (8)$$

where,  $\beta$  is a parameter that considers the soil particle's compactness, particle roughness, and porous media extent. The  $\beta$  values for different permeameter diameters are:

Permeameter diameter (cm)	$\beta$ values
5.08	1.23
10.16	0.96
15.24	0.82

The observed  $\beta$  suggests that its  $\beta$  value is smaller for maximum and larger for minimum, permeameter size. The outcome of the study infers that the increase in the porous media extent results in an insignificant  $\beta$  value (Zieba 2017).

The values of empirical constants i.e.,  $a_0$ ,  $a_1$ ,  $a_2$ ,  $a_3$ , &  $a_4$  are:

$$a_0 = 0.388, a_1 = 0.0327, a_2 = 0.0013, a_3 = 2.56 \times 10^{-5}, \text{ \& } a_4 = 2 \times 10^{-7}$$

For the  $k$  estimation, the developed model is:

$$k = \lambda * \left[ 2 \times 10^{-7} \left( \frac{\sigma}{d_{10}} \right)^4 - 2.56 \times 10^{-5} \left( \frac{\sigma}{d_{10}} \right)^3 + 0.0013 \left( \frac{\sigma}{d_{10}} \right)^2 - 0.0327 \left( \frac{\sigma}{d_{10}} \right) + 0.388 \right] \quad (9)$$

Table 3 represents the developed model-based and experimentally measured  $k$  values for 15 soil samples.

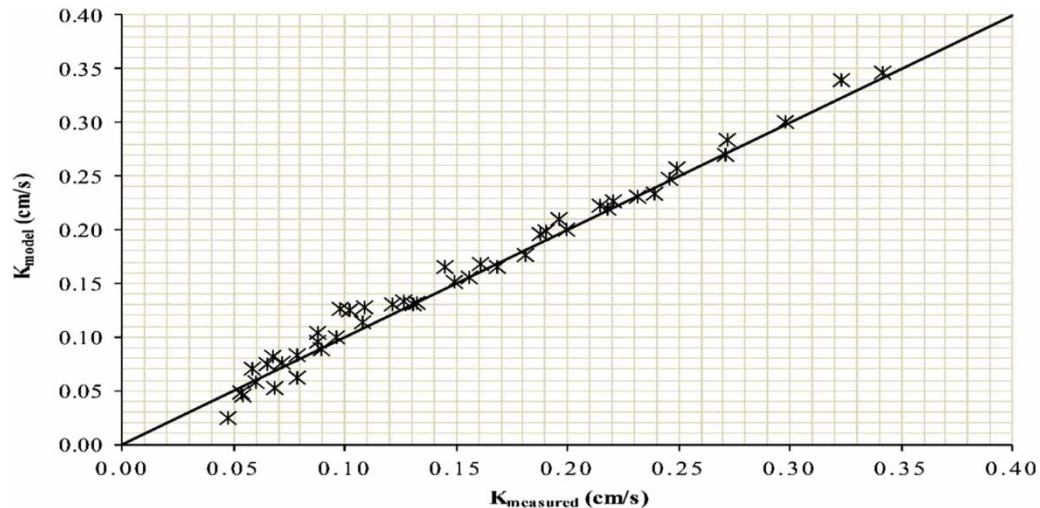
**Table 3** | Developed model-based and measured  $k$  values

Sample no.	$\sigma/d_{10}$	$(k_{\text{measured}})$ Permeameter diameter (cm)			$(k_{\text{model}})$ Permeameter diameter (cm)		
		5.08	10.16	15.24	5.08	10.16	15.24
1	3.805	0.342	0.271	0.232	0.346	0.270	0.231
2	4.390	0.323	0.249	0.219	0.330	0.257	0.220
3	5.539	0.298	0.240	0.200	0.300	0.234	0.200
4	6.669	0.272	0.215	0.188	0.273	0.213	0.182
5	7.876	0.246	0.197	0.169	0.248	0.194	0.165
6	9.058	0.221	0.181	0.150	0.226	0.176	0.151
7	10.787	0.190	0.156	0.132	0.199	0.155	0.132
8	13.255	0.162	0.131	0.109	0.168	0.131	0.112
9	15.104	0.145	0.121	0.097	0.150	0.117	0.100
10	17.376	0.126	0.102	0.088	0.133	0.104	0.089
11	21.754	0.109	0.090	0.072	0.114	0.089	0.076
12	25.964	0.098	0.079	0.065	0.106	0.083	0.071
13	29.868	0.088	0.068	0.059	0.104	0.082	0.070
14	34.467	0.078	0.061	0.053	0.093	0.076	0.064
15	41.066	0.069	0.054	0.048	0.082	0.065	0.052

Further, the experimentally measured  $k$  values have been compared with the developed model-based  $k$  values. Figure 5 indicates a relatively good agreement between the measured and developed model-based  $k$  values.

Further, the uncertainty analysis of the developed model, i.e., Equation (9) has been performed (Parsaie & Haghbiabi 2021). In Equation (9), the dependent parameter is  $k$ , whereas the independent parameters are  $\sigma$  &  $d_{10}$ . Initially, for uncertainty analysis, the  $\sigma$  value remains constant, while the  $d_{10}$  value increases from the baseline value up to 20%, resulting in an increase in the  $k$  value by 5.5%. The  $k$  value decreases up to 6.2%, when the  $\sigma$  value increases from 0 to 20%, while the  $d_{10}$  value remains constant. Based on the results, both the  $\sigma$  and  $d_{10}$  parameters are dependent on  $k$ , while the  $d_{10}$  is positively sensitive to  $k$  and  $\sigma$  is inversely sensitive to  $k$ .





**Figure 5** | Measured and model-based comparison of  $k$ .

### **k** evaluation using empirical models

In this study, four empirical models have been used to evaluate the  $k$  values. Table 2 represents the grain-size parameters that were used to calculate the  $k$  values via empirical models.

From Figure 6, it has been observed that for soil samples, the Hazen model provides the closest fit to the measured  $k$  values, followed by the Kozeny-Carman model. Because it takes into account the grain-size curve, porosity, and effective grain-size, the Hazen model calculates  $k$  more precisely than the other models (Ishaku *et al.* 2011; Chandel & Shankar 2022). Also, as evident in Figure 6, the developed model exhibits reasonably good agreement with the measured  $K$  values when compared to the empirical models investigated in the study.

Further, various statistical indicators were used to evaluate the quantitative assessment of the developed model for different diameter permeameters as given in Table 4.

The BIAS, MAE,  $S_i$ , and RMSE values vary from 0 to  $\infty$  and  $R^2$  and  $I_a$  from 0 to 1 (Rosas *et al.* 2014). Lower values of MAE,  $S_i$ , BIAS, and RMSE, and values closer to 1 for  $I_a$  and  $R^2$  point to a better agreement between the observed and measured parameters (Naej *et al.* 2017). The BIAS,  $S_i$ , MAE, RMSE,  $R^2$ , and  $I_a$  values for the developed model are 0.006, 0.039, 0.005, 0.009, 0.978, and 0.976 for 5.08 cm, 0.003, 0.051, 0.008, 0.005, 0.984, and 0.974 for 10.16 cm, and 0.004, 0.041, 0.007, 0.006, 0.962, and 0.981 for 15.24 cm diameter permeameters respectively. The values of statistical indicators for the developed model establish its efficacy in estimating the  $k$  of porous media.

### **Validation of the grain-size model**

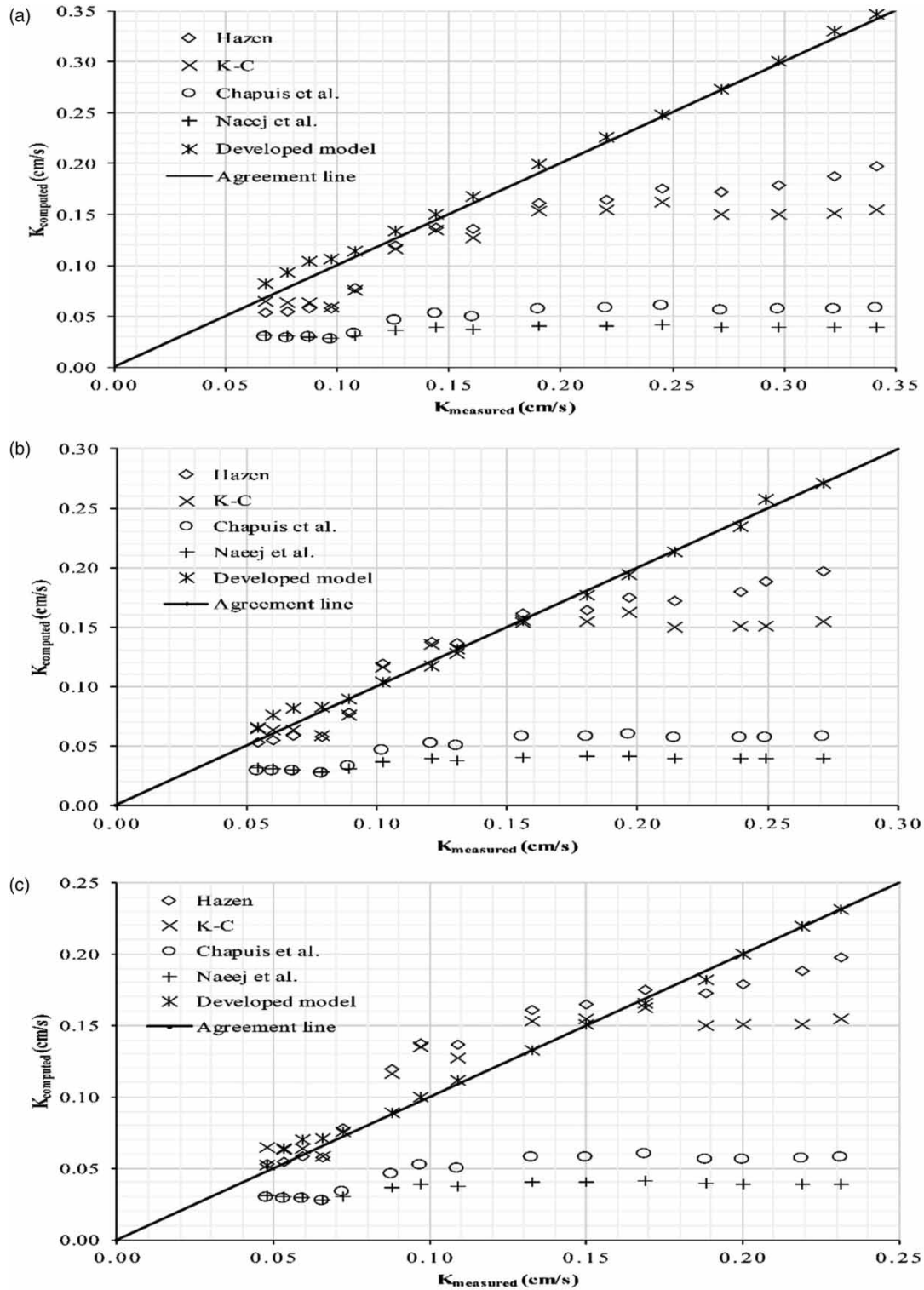
The grain-size parameters ( $d_{10}$  and  $\sigma$ ) of the remaining 12 soil samples have been determined and used for validation. For computing the  $k$  value, the developed model includes a parameter, i.e.,  $\sigma/d_{10}$ . Therefore, for these 12 soil samples, the  $\sigma/d_{10}$  values have been determined as shown in Table 5.

For validation, the hydraulic conductivity for the 12 soil samples has been computed via the grain-size model and then compared with the permeameter-based measured values as shown in Figure 7.

Figure 7 indicates a fairly good agreement between the measured and developed model-based predicted  $k$  values. For the developed model the BIAS,  $I_a$ ,  $R^2$ ,  $S_i$ , RMSE, and MAE values during validation are 0.038, 0.885, 0.934, 0.048, 0.016, and 0.028 respectively, which substantiate the performance of the grain-size model in estimating the hydraulic conductivity of porous media.

## **CONCLUSIONS**

The present study is focused on developing a hydraulic conductivity model using grain-size parameters i.e.,  $d_{10}$  and  $\sigma$ . The grain-size model comprises a factor ' $\beta$ ' which inherits the particle roughness, porous media extent,



**Figure 6** | Evaluation of k values for (a) 5.08, (b) 10.16, & (c) 15.24 cm permeameters size.

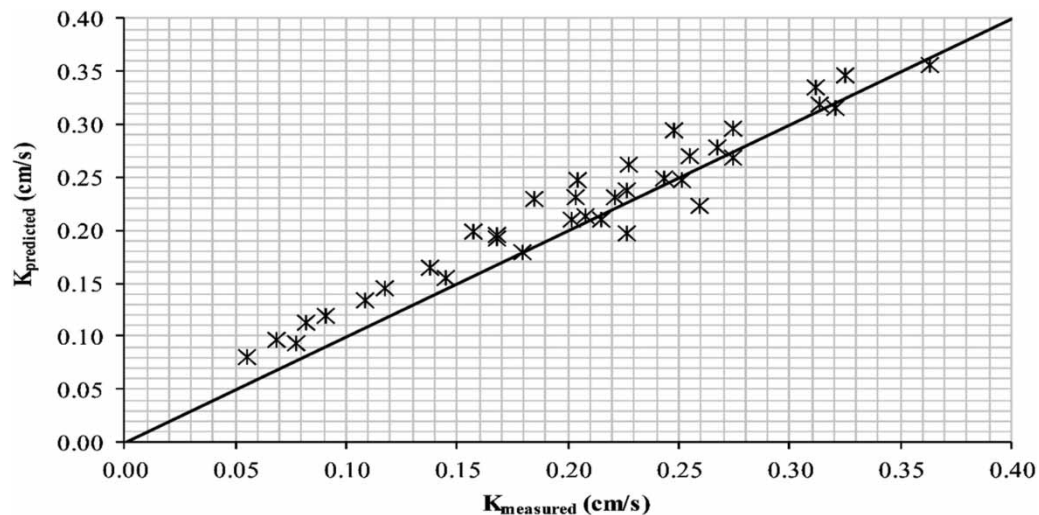
and compactness of porous media. The increase in the porous media extent results in an insignificant  $\beta$  value. For borehole soil samples, the influence of  $d_{10}$  and  $\sigma$  on the hydraulic conductivity elucidate that the  $k$  increases with the increase in the  $d_{10}$  grain-size and decreases with the increase in the  $\sigma$  value. The evaluation of  $k$  values via empirical models infers that the Hazen model performs well in  $k$  estimation, followed by the Kozeny-Carman

**Table 4** | Grain-size model quantitative assessment via statistical indicators

Permeameter Diameter (cm)	Statistical indicators					
	BIAS	S <sub>i</sub>	R <sup>2</sup>	I <sub>a</sub>	MAE	RMSE
5.08	0.006	0.039	0.978	0.976	0.005	0.009
10.16	0.003	0.051	0.984	0.974	0.008	0.005
15.24	0.004	0.041	0.962	0.981	0.007	0.006

**Table 5** | Predicted k via the grain-size model

Sample no.	$\sigma/d_{10}$	(k <sub>predicted</sub> )			(k <sub>measured</sub> )		
		Permeameter diameter (cm)			Permeameter diameter (cm)		
		5.08	10.16	15.24	5.08	10.16	15.24
1	3.519	0.355	0.277	0.236	0.364	0.268	0.227
2	3.831	0.345	0.270	0.230	0.325	0.255	0.222
3	4.800	0.319	0.249	0.212	0.314	0.244	0.208
4	5.714	0.295	0.231	0.197	0.275	0.204	0.227
5	4.930	0.315	0.246	0.210	0.321	0.252	0.202
6	4.215	0.335	0.261	0.223	0.312	0.228	0.260
7	5.818	0.293	0.229	0.195	0.248	0.185	0.168
8	6.920	0.268	0.209	0.178	0.275	0.215	0.180
9	7.982	0.246	0.192	0.164	0.205	0.168	0.138
10	10.792	0.199	0.155	0.132	0.158	0.145	0.109
11	15.850	0.144	0.112	0.096	0.118	0.082	0.069
12	20.160	0.119	0.093	0.080	0.091	0.078	0.056

**Figure 7** | Comparison between measured and predicted k.

model. The quantitative assessment of the grain-size model via the statistical indicators substantiates its efficacy in estimating the k. Also, using the independent data set, the study validates the performance of the grain-size model in estimating k. The study recommends further investigations to examine the influence of particle and wall roughness on the k of porous media.

## DATA AVAILABILITY STATEMENT

All relevant data are included in the paper or its Supplementary Information.

## CONFLICT OF INTEREST

The authors declare there is no conflict.

## REFERENCES

- ASTM 2006 *Standard D2434 – Permeability of Granular Soils (Constant Head)*. West Conshohocken, PA, USA.
- Cabalar, A. F. & Akbulut, N. 2016 Effects of the particle shape and size of sands on the hydraulic conductivity. *Acta Geotechnica Slovenica* **13**(2), 83–93.
- Carman, P. C. 1937 Fluid flow through granular beds. *Transactions Institute of Chemical Engineers* **15**, 150–166.
- Carman, P. C. 1956 *Flow of Gases Through Porous Media*. Butterworths Scientific Publications, London.
- Chandel, A. & Shankar, V. 2021 [Evaluation of empirical relationships to estimate the hydraulic conductivity of borehole soil samples](#). *ISH Journal of Hydraulic Engineering* 1–10.
- Chandel, A. & Shankar, V., 2022 Assessment of hydraulic conductivity of porous media using empirical relationships. In: *Sediment Transport* (Pasquali, D., ed.). IntechOpen, London. doi:10.5772/intechopen.103127.
- Chandel, A., Shankar, V. & Alam, M. A. 2021 [Experimental investigations for assessing the influence of fly ash on the flow through porous media in Darcy regime](#). *Water Science and Technology* **83**(5), 1028–1038.
- Chandel, A., Fayaz, F. & Shankar, V. 2022a [Assessment of column to particle diameter ratio on the hydraulic conductivity of porous media: Wall effect in Darcy Regime](#). *ISH Journal of Hydraulic Engineering* 1–9.
- Chandel, A., Sharma, S. & Shankar, V. 2022b [Prediction of hydraulic conductivity of porous media using a statistical grain-size model](#). *Water Supply* **22**(4), 4176–4192.
- Chapuis, R. P., Allaire, V., Marcotte, D., Chouteau, M., Acevedo, N. & Gagnon, F. 2005 [Evaluating the hydraulic conductivity at three different scales within an unconfined sand aquifer at Lachenalia, Quebec](#). *Canadian Geotechnical Journal* **42**(4), 1212–1220.
- Hazen, A. 1892 Some Physical Properties of Sands and Gravels, With Special Reference to Their Use in Filtration. *Massachusetts State Board of Health, 24th Annual Report*, 539–556.
- Ishaku, J. M., Gadzama, E. W. & Kaigama, U. 2011 Evaluation of empirical formulae for the determination of hydraulic conductivity based on grain-size analysis. *Journal of Geology and Mining Research* **3**(4), 105–113.
- Khaja, M. A., Shah, S. R. & Jha, R. 2022 [Evaluation of empirical models for estimating hydraulic conductivity using gradation characteristics of unconsolidated fluvial sediments](#). *Arabian Journal of Geosciences* **15**(8), 1–17.
- Kozeny, J. 1927 Via capillary conduit the water in the ground. *R. Acad. Sci. Vienna Proc. Class I.* **136**, 271–306.
- Kundu, P., Kumar, V. & Mishra, I. M. 2016 [Experimental and numerical investigation of fluid flow hydrodynamics in porous media: characterization of pre-Darcy, Darcy and non-Darcy flow regimes](#). *Powder Technology* **303**, 278–291.
- Leroueil, S., Le Bihan, J. P., Sebaihi, S. & Alicescu, V. 2002 Hydraulic conductivity of compacted tills from northern Quebec. *Canadian Geotechnical Journal* **39**(5), 1039–1049.
- Lu, C., Chen, X., Cheng, C., Ou, G. & Shu, L. 2012 [Horizontal hydraulic conductivity of shallow streambed sediments and comparison with the grain-size analysis results](#). *Hydrological Processes* **26**(3), 454–466.
- Mujtaba, H., Shimobe, S., Farooq, K. & Khalid, U. 2021 [Relating gradational parameters with hydraulic conductivity of sandy soils: a renewed attempt](#). *Arabian Journal of Geosciences* **14**(18), 1–17.
- Naeef, M., Naeef, M. R., Salehi, J. & Rahimi, R. 2017 [Hydraulic conductivity prediction based on grain-size distribution using M5 model tree](#). *Geomechanics and Geoengineering* **12**(2), 107–114.
- Parsaie, A. & Haghbi, A. H. 2021 [Uncertainty analysis of discharge coefficient of circular crested weirs](#). *Applied Water Science* **11**(2), 1–6.
- Parvazinia, M., Nassehi, V., Wakeman, R. J. & Ghoreishy, M. H. R. 2006 [Finite element modelling of flow through a porous medium between two parallel plates using the Brinkman equation](#). *Transport in Porous Media* **63**(1), 71–90.
- Pliakas, F. & Petalas, C. 2011 [Determination of hydraulic conductivity of unconsolidated river alluvium from permeameter tests, empirical formulas and statistical parameters effect analysis](#). *Water Resources Management* **25**(11), 2877–2899.
- Pucko, T. & Verbovsek, T. 2015 [Comparison of hydraulic conductivities by grain-size analysis, pumping, and slug tests in Quaternary gravels, NE Slovenia](#). *Open Geosciences* **1**, 308–317.
- Ren, X. W. & Santamarina, J. C. 2018 [The hydraulic conductivity of sediments: a pore size perspective](#). *Engineering Geology* **233**, 48–54.
- Rosas, J., Lopez, O., Missimer, T. M., Coulibaly, K. M., Dehwah, A. H., Sesler, K., Lujan, L. R. & Mantilla, D. 2014 [Determination of hydraulic conductivity from grain-size distribution for different depositional environments](#). *Groundwater* **52**(3), 399–413.
- Rushton, K. R. 2004 *Groundwater Hydrology: Conceptual and Computational Models*. John Wiley & Sons, Chichester, West Sussex, UK.
- Salarashayeri, A. F. & Siosemarde, M. 2012 [Prediction of soil hydraulic conductivity from particle-size distribution](#). *International Journal of Geological and Environmental Engineering* **6**(1), 16–20.
- Singh, B., Sihag, P., Parsaie, A. & Angelaki, A. 2021 [Comparative analysis of artificial intelligence techniques for the prediction of infiltration process](#). *Geology, Ecology, and Landscapes* **5**(2), 109–118.

- Toumpanou, I. C., Pantazopoulos, I. A., Markou, I. N. & Atmatzidis, D. K. 2021 Predicted and measured hydraulic conductivity of sand-sized crushed limestone. *Bulletin of Engineering Geology and the Environment* **80**(2), 1875–1890.
- Wang, J. P., François, B. & Lambert, P. 2017 Equations for hydraulic conductivity estimation from particle size distribution: a dimensional analysis. *Water Resources Research* **53**(9), 8127–8134.
- Zieba, Z. 2017 Influence of soil particle shape on saturated hydraulic conductivity. *Journal of Hydrology and Hydromechanics* **65**(1), 80–87.

First received 11 April 2022; accepted in revised form 16 August 2022. Available online 23 August 2022