

## Cation interception and permeability characteristics of bentonite barriers exposed to NaCl and NH<sub>4</sub>Cl solutions

Wen-jing Sun<sup>a,b,\*</sup>, Qian-tong Tang<sup>a</sup>, Ri-dong Fan<sup>a</sup>, Ai-wu Yang<sup>a</sup>, Yun-zi Tan<sup>c</sup> and A. K. Leung<sup>d</sup>

<sup>a</sup> College of Environmental Science and Engineering, Donghua University, Shanghai 201620, China

<sup>b</sup> Key Laboratory of Ministry of Education on Safe Mining of Deep Metal Mines, Northeastern University, Shenyang 110819, China

<sup>c</sup> College of Civil Engineering and Architecture, China Three Gorges University, Yichang 443002, China

<sup>d</sup> Department of Civil and Environmental Engineering, Hong Kong University of Science and Technology, Clear Water Bay, Kowloon, Hong Kong, China

\*Corresponding author. E-mail: wjsun@dhu.edu.cn

### ABSTRACT

High concentrations of Na<sup>+</sup> and NH<sub>4</sub><sup>+</sup> in landfill leachate lead to deterioration of bentonite barrier and pose a threat to the environment. This study focused on the pollution interception and permeability characteristics of the bentonite barrier exposed to NaCl and NH<sub>4</sub>Cl solutions. Based on previous findings, salt solution concentrations were established at 74.80, 37.40, 18.70, and 9.4 mmol/L. The bentonite contents in the mixture were set at 0, 5, 10, and 15%. The results indicate that the samples exhibit better interception of NH<sub>4</sub><sup>+</sup> compared to Na<sup>+</sup>. This difference arises from the cation exchange sequence, the size of the hydration radius, and the hydrogen bonding of the two cations. Additionally, the difference in hydration enthalpy between the two cations leads to variations in the swelling of bentonite, resulting in a higher hydraulic conductivity coefficient in NH<sub>4</sub>Cl solution. This study shows that although bentonite barriers have better interception for NH<sub>4</sub><sup>+</sup>, they exhibit greater hydraulic conductivity in NH<sub>4</sub>Cl solution, increasing the risk of leachate carrying other contaminants.

**Key words:** bentonite, cation exchange, cation interception, permeability characteristics, swelling index

### HIGHLIGHTS

- Compared permeability and residual leachate of bentonite barriers in NaCl and NH<sub>4</sub>Cl solutions.
- Bentonite intercepts NH<sub>4</sub><sup>+</sup> more effectively than Na<sup>+</sup>.
- In NH<sub>4</sub>Cl, bentonite has lower swelling and higher hydraulic conductivity than in NaCl.
- Variations in cation exchange, hydration radius, and bonding explain the differences.
- Hydration enthalpy differences lead to varying hydraulic conductivities in the solutions.

## 1. INTRODUCTION

Currently, urban solid waste management heavily relies on landfilling to dispose of solid wastes that cannot be recycled or incinerated. In China, approximately 79% of urban solid waste is disposed of in landfills (Havukainen *et al.* 2017).

High volume of leachate will be produced due to complex physical and chemical reactions during landfill operation. Na<sup>+</sup> and NH<sub>4</sub><sup>+</sup> constitute significant inorganic components, often present in high concentrations (Costa *et al.* 2019; Luo *et al.* 2020). It was found that NH<sub>4</sub><sup>+</sup> in leachate increased over time, and the concentration can be 2,000 mg/L (Negi *et al.* 2020). Within 1–2 years of operation in a landfill, the concentration of NH<sub>4</sub><sup>+</sup> can reach up to 4,000 mg/L (Sun *et al.* 2021). According to survey statistics, the average concentration of Na<sup>+</sup> in leachate is higher than that of common inorganic components such as K<sup>+</sup> and Ca<sup>2+</sup> (averaging 1,675 and 400 mg/L, respectively), as well as other heavy metals (Cu<sup>2+</sup>, Zn<sup>2+</sup>, Pb<sup>2+</sup>, etc.). The highest concentration approaches 4,000 mg/L (3,710 mg/L) (Naveen *et al.* 2017). As a result, leachate resulting from MSW landfills is a typical source of excessive NH<sub>4</sub><sup>+</sup> and Na<sup>+</sup>, and these should be main cations considered for landfill design. Na<sup>+</sup> and NH<sub>4</sub><sup>+</sup> have little effect on bentonite engineering properties in low concentrations (80 mM) (Setz *et al.* 2017), but high value will affect the performance of bentonite barriers such as its morphological structure, swelling index (SI), hydraulic conductivity, and pollution interception (Anh *et al.* 2017; Setz *et al.* 2017). It may even affect the acidity and alkalinity of the environment (Lai *et al.* 2023).

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Geosynthetic clay liners (GCLs) are often used in landfills to prevent the migration of potential pollution due to landfill leachate (Özçoban *et al.* 2022). However, both  $\text{Na}^+$  and  $\text{NH}_4^+$  are monovalent cations. Generally, in comparison to divalent cations, monovalent cations possess larger diffusion coefficients (Tadimeti & Chattopadhyay 2016; Huang *et al.* 2017; Aryal & Ganesan 2018; Dong *et al.* 2020). This facilitates their movement within the landfill liner. Further investigation is warranted to elucidate the variance in pollution interception capacity of bentonite for  $\text{Na}^+$  and  $\text{NH}_4^+$  present in landfill leachate. Additionally, the impact of  $\text{Na}^+$  and  $\text{NH}_4^+$  on the permeability properties of bentonite-based engineering barriers remains a subject that requires deeper scrutiny. Clarifying the distinctions in their respective influences is imperative. This study contributes to a better understanding of the behavior of leachate pollutants within landfill barriers, particularly focusing on the  $\text{Na}^+$  and  $\text{NH}_4^+$  retention in bentonite and the permeability alterations of bentonite.

To this end, mixture soil samples are prepared using bentonite and clay in this study, and the contents of bentonite are set at 0, 5, 10, 15, 20%, respectively. NaCl and  $\text{NH}_4\text{Cl}$  solutions with four different concentrations were used to permeate bentonite–clay mixture samples under a constant pressure. The leachate concentration of  $\text{Na}^+$  and  $\text{NH}_4^+$  was obtained. Fourier transform infrared (FTIR) analysis was performed to understand the interaction between  $\text{NH}_4^+$  and bentonite. The effects of bentonite content and initial concentration on residual leachate concentration of  $\text{Na}^+$  and  $\text{NH}_4^+$  were investigated. The hydraulic conductivity coefficients of bentonite are obtained, and the SI of bentonite was tested. The relationship between swell potential and hydraulic conductivity coefficients of bentonite exposed to NaCl and  $\text{NH}_4\text{Cl}$  solution with different concentrations were analyzed.

## 2. METHODS

### 2.1. Materials

Geosynthetic clay liner (GCL) bentonite (abbr. bentonite) and natural clay obtained from Shanghai (abbr. clay) were used in this study. The main clay mineral of the bentonite is montmorillonite that accounts for 45.8% of clay mineral according to X-ray diffraction analyses. The basic physical properties of the bentonite and clay were tested as per ASTM D4318-10, as shown in Table 1. The bentonite is classified as a fat clay (CH).

NaCl and  $\text{NH}_4\text{Cl}$  solutions were prepared using analytically pure (AR99.5%) NaCl and  $\text{NH}_4\text{Cl}$  powders (analytical reagent, AR) obtained from Sinopharm Chemical Reagent Co., Ltd. Based on previous tests (Lou *et al.* 2007; Sun *et al.* 2021), the concentration of  $\text{NH}_4\text{Cl}$  solution is set at 9.4, 18.7, 37.4, and 74.8 mmol/L (It is equivalent to 500, 1,000, 2,000, and 4,000 mg/L respectively), corresponding to the residual  $\text{NH}_4^+$  concentration after 2, 4, 6, and 8 years of operation in the landfill, respectively. It is reported that the concentrations of  $\text{Na}^+$  were comparable to that of  $\text{NH}_4^+$  in landfill leachate (Gupta & Paulraj 2017). Thus, the concentrations of NaCl were controlled to be equal to the concentrations of  $\text{NH}_4\text{Cl}$ .

### 2.2. Preparation of samples

The bentonite and clay were dried at 105 °C for 24 h. Subsequently, the dried bentonite was thoroughly mixed with dry clay powder to prepare a homogeneous dry mixture. The dry mixture comprised varying proportions of bentonite and clay, specifically 0, 5, 10, and 15%. Deionized water (DI water) was added to the dry mixture to achieve a mass ratio of water to dry mixture of 1:5. The mixture was rigorously mixed to ensure uniformity, and the resulting mixture samples were placed in

**Table 1** | Basic physical indexes of bentonite and clay

Physical property	Bentonite	Natural clay
Liquid limit, $w_L$ (%)	153.4	36.0
Plastic limit, $w_p$ (%)	26.8	22.2
Plasticity index, $I_p$	126.6	13.8
Specific gravity ( $G_s$ )	2.7	2.7
Swelling index, $SI$ (mL/2 g)	28.7	N.D.
Cation exchange capacity, $CEC$ (meq/100 g)	68.0	N.D.
$D_{50}$ ( $\mu\text{m}$ )	7.2	7.5
$D_{97}$ ( $\mu\text{m}$ )	44.5	29.3

N.D., not determined.

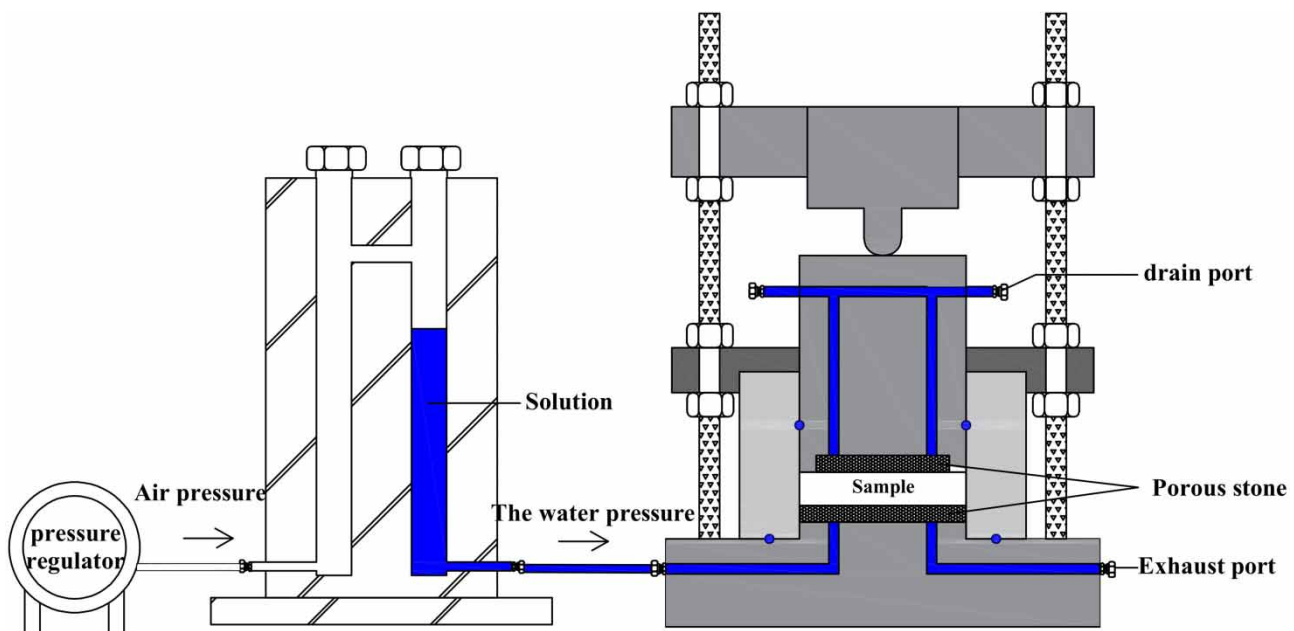
sealed bags and left to stand for 24 h at 20 °C. Then, they were compacted using a jack to form samples with a diameter of 3.8 cm, a height of 1.5 cm, a specific gravity of 2.68 ( $\pm 0.01$ ) g/cm<sup>3</sup>, and a water content of 20% ( $\pm 0.5\%$ ). In the saturated permeability test, the initial moisture content of the sample has modest influence on the test results, and the initial moisture content of 20% is set to make the soil sample reach a suitable plastic state in order to prepare samples conveniently. This procedure was repeated to produce a total of 36 samples with varying bentonite contents. The samples used in the outflow measurement test are summarized as shown in Table 2.

### 2.3. Permeability test

The permeability test was conducted using an instrument of controlling water head pressure, as illustrated in Figure 1. Referring to the high hydraulic pressure tests (400 kPa) conducted by scholars such as Sun *et al.* 2021 and Chen *et al.* 2018 to shorten the testing time for low-permeability materials. The instrument is designed to create head pressure in the solution using air pressure.

**Table 2** | The test scheme of outflow measurement

Type of solution	Dry density $\rho_d$ (g/cm <sup>3</sup> )	Moisture content $w$ (%)	Bentonite content $a$ (%)	Concentration $C$ (mmol/L)
DI water	1.7	20	0	/
			5	/
			10	/
			15	/
NaCl			0	9.4
			5	18.7
			10	37.4
			15	74.8
NH <sub>4</sub> Cl			0	9.4
			5	18.7
			10	37.4
			15	74.8



**Figure 1** | Illustration of the constant head system.

The sample was installed on the instrument, with soaked filter papers and permeable stones placed at the upper and lower ends of the sample, respectively. Vaseline was applied to the inner walls of the instrument to ensure sealing. After fixing the sample in place, the appropriate permeation solution (NaCl or NH<sub>4</sub>Cl) was introduced into the salt separator while ensuring the salt separator remained closed. Once the solution was added, the leachate collector was connected to the instrument. Subsequently, the air pressure valve was opened, and 400 kPa of air pressure was applied for a specified duration to remove any residual gas from the permeameter. During testing of each sample, the volume of the leachate found in the collector was continuously monitored and a relationship between time (s) and leachate volume (cm<sup>3</sup>) can be obtained. When the leachate volume change rate becomes stable, it is considered that the sample has reached saturation and the permeability has reached a stable state. Finally, the permeation flux  $Q$  (cm<sup>3</sup>/s) through the stable stage can be obtained and the hydraulic conductivity coefficient of the sample can be determined by Equation (1).

$$k = \frac{QL}{A\Delta h} \quad (1)$$

where  $k$  is the permeation coefficient of the sample;  $Q$  is the stable permeation flux (cm<sup>3</sup>/s);  $L$  is the height of the sample (cm);  $A$  is the bottom area of the soil sample (cm<sup>2</sup>);  $\Delta h$  is the hydraulic head difference (cm).

When the rate of change in permeation flux  $Q$  for three consecutive measurements is less than 1%, it is considered that permeation has reached equilibrium, and the permeability coefficient of the sample is obtained.

#### 2.4. The residual leachate concentration tests

After obtaining stable hydraulic conductivity coefficients in the permeation tests described in Section 2.3, the leachate was obtained. The Na<sup>+</sup> concentration in the leachate was determined through Inductively Coupled Plasma (ICP) testing. The NH<sub>4</sub><sup>+</sup> concentration in the leachate was determined through Ion Chromatography (IC) testing.

#### 2.5. SI test

Bentonite exhibits sensitive swelling behavior in cationic solutions. To conduct targeted research, the SI of bentonite was tested following ASTM D5890 standards. This test aimed to assess how the swelling potential of bentonite changes with varying Na<sup>+</sup>/NH<sub>4</sub><sup>+</sup> concentrations. Each sample was tested in duplicate. The test scheme of SI is presented in Table 3. DI water was also used for comparative purposes.

Following the procedure outlined in ASTM D5890, not more than 0.1 g increments of bentonite were removed and were evenly distributed over the water surface in a graduated cylinder within a span of approximately 30 s. Additional bentonite was added continuously at 10-min intervals, ensuring that each increment swells without entrapping air between them, until the entire bentonite sample has been added. The bentonite sample was allowed to settle for 24 h from the last addition, and the volume level in mL was recorded to the nearest 0.5 mL at the top of the settled clay mineral.

#### 2.6. FTIR analysis

FTIR analysis was applied to identify specific functional groups in the mixture samples containing NaCl and NH<sub>4</sub>Cl solutions with concentration of 37.4 mmol/L and DIW. Fourier infrared spectrometer Nicolet6700 was used to record the spectra ranging from 4,000 to 400 cm<sup>-1</sup> at a spectral resolution of 4 cm<sup>-1</sup>. Scan repetition was 32 times. The sample used for FTIR analysis was prepared by mixing sample with KBr with a mass ratio of approximately 1–50. The sample name, bentonite content, type of solutions as well as its concentration are summarized in Table 4.

**Table 3** | The test scheme of SI

Bentonite content (%)	Bentonite mass (g)	Type of solution	Concentration C (mmol/L)
5	1.45	NaCl	0
10	2.89	NH <sub>4</sub> Cl	9.4
15	4.34		18.7
			37.4
			74.8

**Table 4** | The test scheme of FTIR

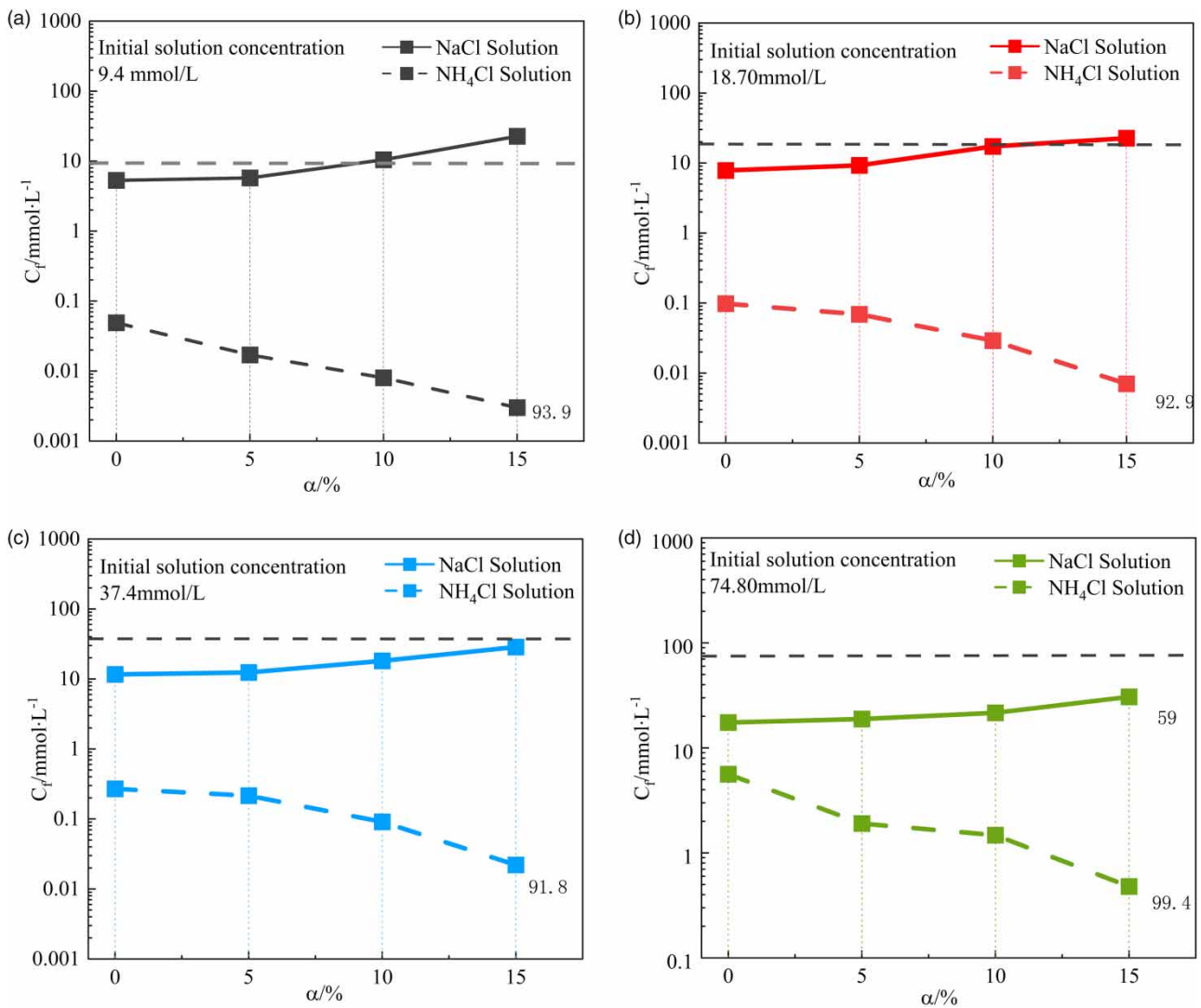
Samples name	Bentonite content, $\alpha$ (%)	Type of solution	Concentration, $C$ (mmol/L)
Na-sample	5	NaCl	37.4
NH <sub>4</sub> -sample	5	NH <sub>4</sub> Cl	37.4
DIW-sample	5	DIW	-

### 3. RESULTS

#### 3.1. The residual leachate concentration of Na<sup>+</sup>/NH<sub>4</sub><sup>+</sup>

Figure 2(a)–2(d) shows the variations between Na<sup>+</sup>/NH<sub>4</sub><sup>+</sup> concentration of leachate through the mixture sample and bentonite content when mixture samples were subjected to NaCl and NH<sub>4</sub>Cl solutions with concentration of 9.4, 18.7, 37.4, 74.8 mmol/L, respectively.

The result indicates that the NH<sub>4</sub><sup>+</sup> concentrations in leachate significantly decrease with increasing bentonite content, indicating an effective containment of NH<sub>4</sub><sup>+</sup> in leachate. The reduction in the concentration can be as high as 99% compared with



**Figure 2** | Variations of the concentration of Na<sup>+</sup>/NH<sub>4</sub><sup>+</sup> in leachate over bentonite contents under leachate of NaCl and NH<sub>4</sub>Cl solutions. (a)  $C = 9.4$  mmol/L, (b)  $C = 18.7$  mmol/L, (c)  $C = 37.4$  mmol/L, (d)  $C = 74.8$  mmol/L.

its corresponding initial concentration. For example, as the bentonite content increases from 0 to 15%, the  $\text{NH}_4^+$  concentrations in leachate decreases from 5.61 to 0.48 mmol/L when mixture samples subjected to  $\text{NH}_4\text{Cl}$  solution with initial concentration of 74.8 mmol/L.

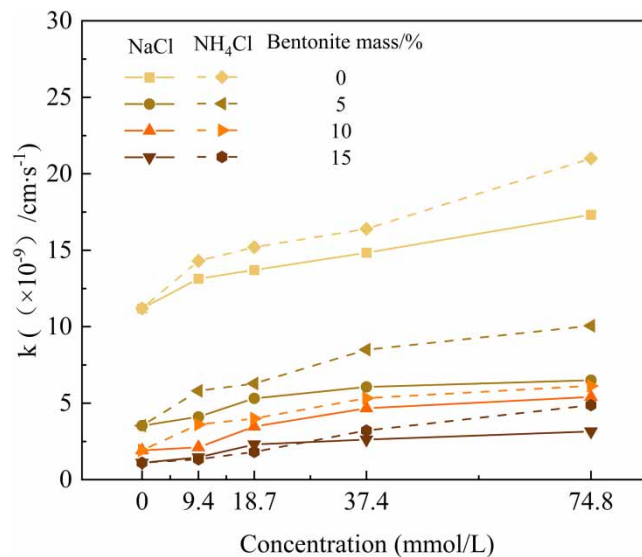
In contrast, the variation in  $\text{Na}^+$  concentration in leachate with bentonite content shows an opposite trend. The  $\text{Na}^+$  concentration in leachate increases with an increase in bentonite content. It should be noted that the  $\text{Na}^+$  concentration in leachate exceeds its initial concentration for mixture samples containing 15% bentonite, permeated with  $\text{NaCl}$  solutions initially at 9.4 and 18.7 mmol/L, as shown in Figure 2(a) and 2(b). The concentrations increase to 22.58 and 22.6 mmol/L, respectively. In addition, it is found that  $\text{Na}^+$  concentration in leachate is generally higher than  $\text{NH}_4^+$  concentration for a given bentonite content.

### 3.2. Permeability characteristics of bentonite in $\text{NaCl}$ solution and $\text{NH}_4\text{Cl}$ solution

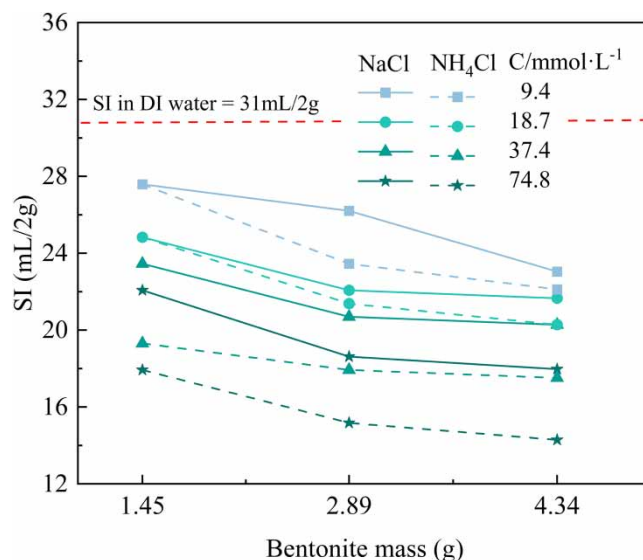
The relationship between the hydraulic conductivity coefficient of mixed soil samples and the bentonite content at different  $\text{Na}^+/\text{NH}_4^+$  concentrations is shown in Figure 3. As seen from the figure, the mixed soil samples exhibit the lowest hydraulic conductivity coefficient at a concentration of 0 mmol/L (DI water), and the hydraulic conductivity coefficient increases with the concentration of  $\text{NaCl}$  and  $\text{NH}_4\text{Cl}$  solutions. Notably, the mixed soil demonstrates different hydraulic conductivity characteristics in  $\text{NaCl}$  and  $\text{NH}_4\text{Cl}$  solutions, with the hydraulic conductivity coefficient generally being higher in  $\text{NH}_4\text{Cl}$  solutions than in  $\text{NaCl}$  solutions. In mixed soil samples with 5% bentonite content, this difference reaches up to 1.5 times (at a solution concentration of 74.8 mmol/L). Additionally, pure clay shows the highest hydraulic conductivity coefficient in deionized water, and as the bentonite content increases, the hydraulic conductivity coefficient of the mixed soil samples decreases from  $11.2 \times 10^{-9}$  cm/s to  $1.1 \times 10^{-9}$  cm/s. This trend is observed in both  $\text{NaCl}$  and  $\text{NH}_4\text{Cl}$  solutions.

### 3.3. SI of bentonite on $\text{NaCl}$ solution and $\text{NH}_4\text{Cl}$ solution

The SI refers to the increase in the volume of 2 g bentonite due to water absorption without applying any confined pressure (Dutta & Mishra 2015). It not only reflects the swelling potential but also the permeability and diffusion of metals (Chen *et al.* 2018). Figure 4 illustrates the relationship between the SI and bentonite masses at different  $\text{Na}^+/\text{NH}_4^+$  concentration. The swelling of bentonite is not inhibited in DI water, with a SI of 31 mL/2 g. Compared to the 2.89 and 4.34 g samples of bentonite, the 1.45 g sample exhibits the highest level of free swelling. This trend remains consistent across solutions with different concentrations. Thereafter, the bentonite sample exhibits lower value of SI as the mass increases. The SI decreases with increasing  $\text{Na}^+/\text{NH}_4^+$  concentration. In 1.45 g of bentonite, as the concentration of  $\text{NaCl}$  solution increases from 9.4 to 74.8 mmol/L, the SI decreases from 27.6 mL/2 g to 22.1 mL/2 g. Similarly, as the concentration of  $\text{NH}_4\text{Cl}$  solution increases



**Figure 3** | Relationship between bentonite hydraulic conductivity coefficient and bentonite masses at different  $\text{Na}^+/\text{NH}_4^+$  concentrations.



**Figure 4** | Relationship between the SI and bentonite masses at different  $\text{Na}^+/\text{NH}_4^+$  concentration.

from 9.4 to 74.8 mmol/L, the SI decreases from 27.6 mL/2 g to 17.9 mL/2 g. Bentonite exhibits differential swelling behavior in NaCl and  $\text{NH}_4\text{Cl}$  solutions, with lower free swelling observed in  $\text{NH}_4\text{Cl}$  solution.

### 3.4. FTIR

Figure 5 shows the resulting FTIR spectra of mixture samples with bentonite content of 5% exposed to DIW, and NaCl and  $\text{NH}_4\text{Cl}$  solution with concentration of 37.4 mmol/L. In addition, the specific bands of FTIR spectra are summarized in Table 5.

As shown in Figure 5, the band positions at 3,625 and 3,436  $\text{cm}^{-1}$  are due to O–H vibration of cationic binding to octahedron and H–O–H vibration of montmorillonite surface absorbing water, respectively (Kumar & Lingfa 2020). The bands at 1,086 and 1,033  $\text{cm}^{-1}$  are due to Si–O stretching vibration. The bands at 695–471  $\text{cm}^{-1}$  are assigned to Si–O–Al bending vibration.

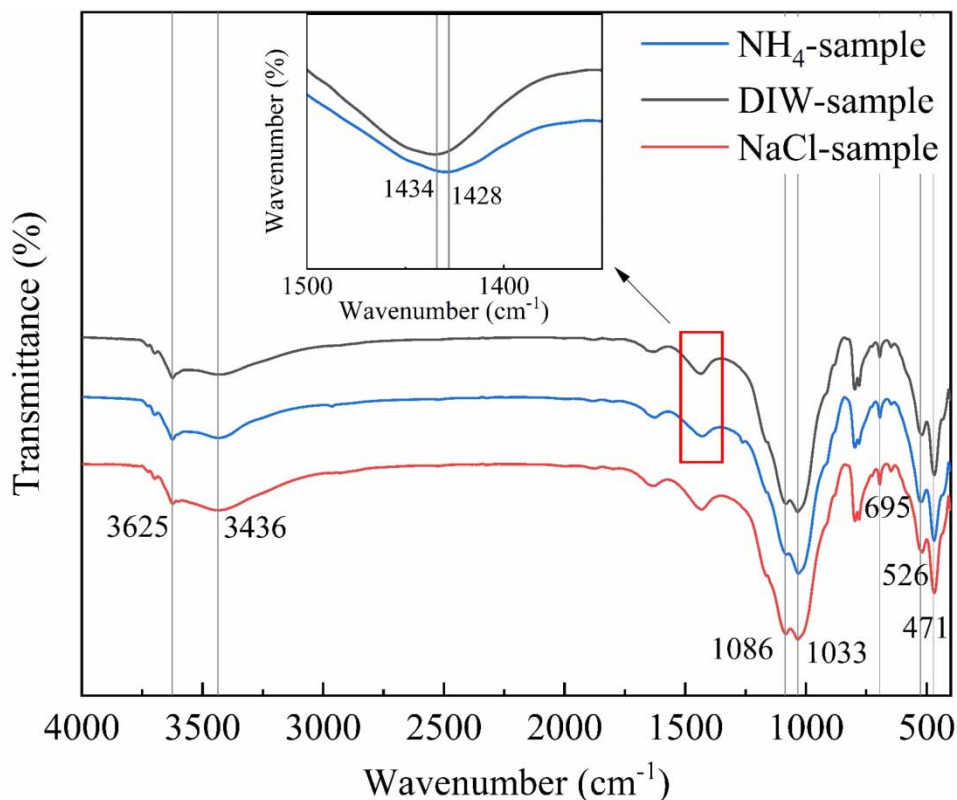
The band at 1,434  $\text{cm}^{-1}$  is due to  $\text{CO}_3^{2-}$  entrainment in the mixture sample (Petit *et al.* 2006). The band offsets to 1,428  $\text{cm}^{-1}$  after exposed to  $\text{NH}_4\text{Cl}$  solution is because the  $\text{NH}_4^+$  bending vibration at 1,427  $\text{cm}^{-1}$  (Gautier *et al.* 2010), indicating that  $\text{NH}_4^+$  is fixed on montmorillonite.

In addition, two bands at 2,963 and 1,263  $\text{cm}^{-1}$  are identified, as shown in Figure 6(a) and 6(b). These bands are located in the  $\text{NH}_4^+$  stretching vibration region (3,300–2,800  $\text{cm}^{-1}$ ) and the  $\text{NH}_4^+$  bending vibration region (1,700–700  $\text{cm}^{-1}$ ), respectively (Gautier *et al.* 2010). Compared to the FTIR spectra of samples in DI water and NaCl solution, new spectral bands appeared in the FTIR spectra of samples in  $\text{NH}_4\text{Cl}$  solution. This is attributed to the interaction between  $\text{NH}_4^+$  and montmorillonite.

## 4. DISCUSSION

In the residual leachate concentration tests, the concentration of  $\text{NH}_4^+$  was significantly lower than that of  $\text{Na}^+$  (as discussed in Figure 2). The decrease in  $\text{NH}_4^+$  concentration of leachate is attributed to the cation exchange between the pollutants and bentonite. The method of ion exchange has been used to remove metals from aqueous solutions (Hussain & Ali 2021). The interlayer cations in the montmorillonite mainly consist  $\text{Na}^+$ ,  $\text{K}^+$ ,  $\text{Ca}^{2+}$  and  $\text{Mg}^{2+}$  (Sun *et al.* 2013). It is reported that the cation replaceability order of bentonite can be as follows:  $\text{NH}_4^+ > \text{Ca}^{2+} > \text{Mg}^{2+} > \text{K}^+ > \text{Na}^+$  (Ye *et al.* 2017; Xiang *et al.* 2020). This indicates that  $\text{NH}_4^+$  in leachate can be fixed more easily due to exchange cation than  $\text{Na}^+$ .

Second, the hydration radius of  $\text{NH}_4^+$  (5.35 Å) is smaller than that of  $\text{Na}^+$  (7.9 Å) (Peng *et al.* 2018). This results in a closer fixture of  $\text{NH}_4^+$  on negatively charged mineral surface than that of  $\text{Na}^+$  (Helle *et al.* 2019). In addition, such hydration radius



**Figure 5** | FTIR spectra of DIW-sample, Na-sample, NH<sub>4</sub>-sample.

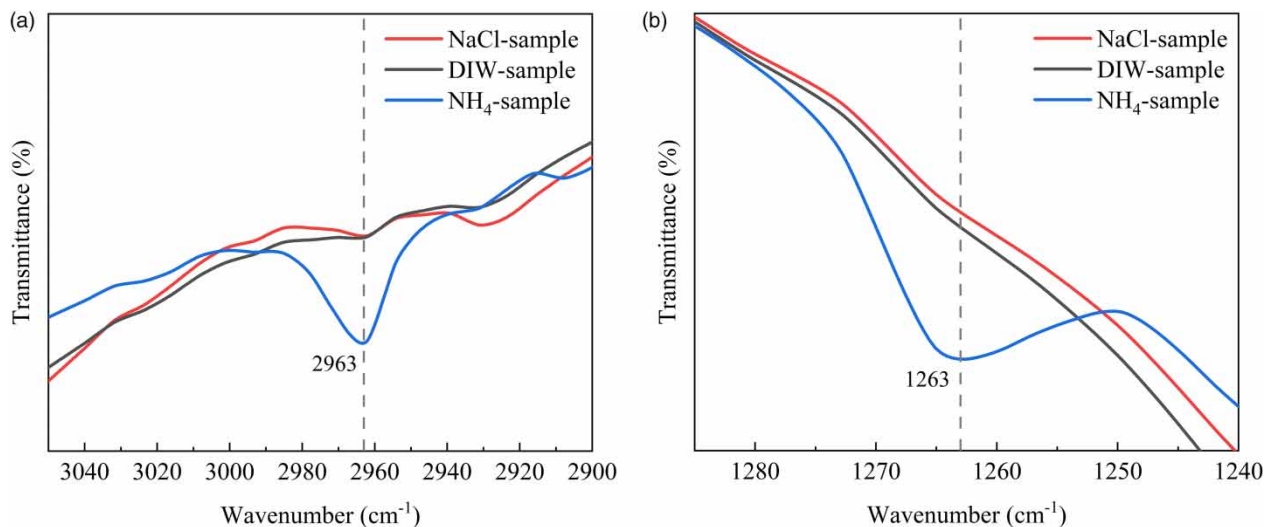
**Table 5** | Band location and assignment of FTIR spectrum

Assignment	Band location, (cm <sup>-1</sup> )	Reference
Symmetrical and asymmetrical O–H bond stretching vibrations of water	3,600–3,200	Gautier <i>et al.</i> (2010), Kumar & Lingfa (2020)
NH <sub>4</sub> <sup>+</sup> stretching vibration	3,300–2,800	Gautier <i>et al.</i> (2010), Petit <i>et al.</i> (2006), Gautier <i>et al.</i> (2010)
NH <sub>4</sub> <sup>+</sup> bending vibration	1,700–700	
Stretching vibration of Si–O	1,200–900	Zazoua <i>et al.</i> (2013)
Bending vibration of Al–O–Si	695.21–471.03	Hussain & Ali (2021)

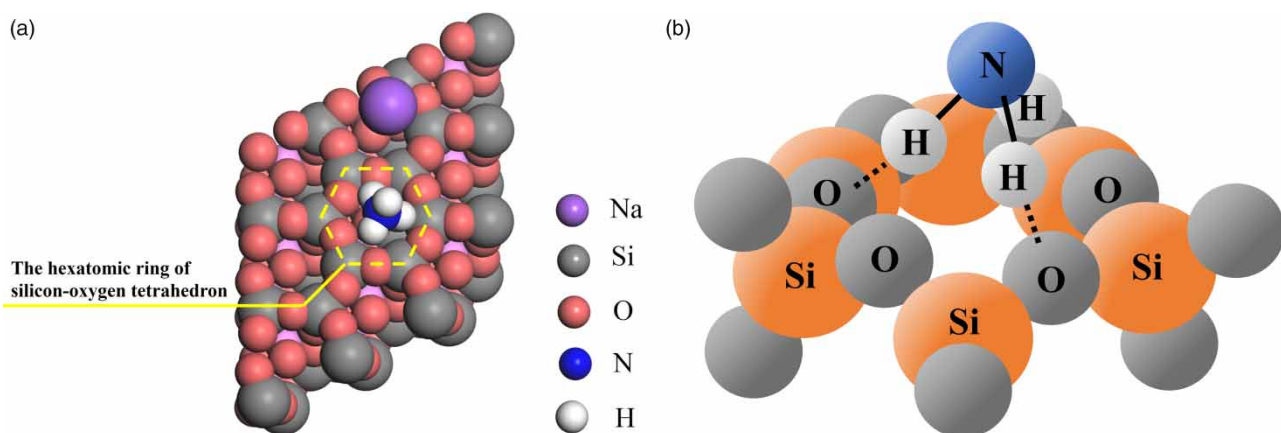
of NH<sub>4</sub><sup>+</sup> allows NH<sub>4</sub><sup>+</sup> to be embedded into the hexatomic ring of silicon–oxygen tetrahedron, which can concatenate the clay layer by hydrogen bonds (N–H...O) between NH<sub>4</sub><sup>+</sup> and silicon hydroxyl groups (Zhang *et al.* 2020). Thus, the NH<sub>4</sub><sup>+</sup> can also be absorbed in inner-sphere complexes above the surface hexagonal cavities, as shown in Figure 7. Therefore, bentonite barriers exhibit good performance in intercepting NH<sub>4</sub><sup>+</sup>. Notably, the Na<sup>+</sup> concentration in leachate increased compared to the initial concentration when a 9.4 mmol/L solution permeated the sample containing 15% bentonite (as seen in Figure 2). This is because the sodium-modified bentonite undergoes a chemical reaction where Na<sup>+</sup> replace Ca<sup>2+</sup>, resulting in some Na<sup>+</sup> occupying the positions of Ca<sup>2+</sup> (Sun *et al.* 2015). Unlike in sodium bentonite, these Na<sup>+</sup> diffuse into the NaCl solution driven by the concentration gradient. In this scenario, the concentration of Na<sup>+</sup> in the effluent may be higher than its initial concentration in the leachate.

The increase in hydraulic conductivity coefficient is due to the reduction in the double-layer thickness of montmorillonite particles caused by Na<sup>+</sup> and NH<sub>4</sub><sup>+</sup>. Positively charged cations appear between the montmorillonite layers, weakening the





**Figure 6** | FTIR spectra of band at  $2,963\text{ cm}^{-1}$  (a) and  $1,263\text{ cm}^{-1}$  (b).



**Figure 7** | Molecular model of  $\text{NH}_4^+$  embedded in the hexameric ring (a), and schematic representation of concatenate the layers by the hydrogen bonds (b) after Casal *et al.* (1984).

repulsion between the layers and the negative charges. This causes the microstructure to transition from a dispersed state to an aggregated state, leading to layer compression (Akinwunmi *et al.* 2020). It leads to an increase in the gaps between particles, reducing water flow resistance, and consequently increasing the hydraulic conductivity. The experimental results of the SI in Section 3.3 also support this view.

In this study, the SI of the mixture sample exposed to both NaCl and  $\text{NH}_4\text{Cl}$  solutions should theoretically be similar for a given concentration, as both  $\text{Na}^+$  and  $\text{NH}_4^+$  are monovalent cations. However, noticeable differences were observed, as shown in Figure 4. The lower SI of the mixture sample exposed to  $\text{NH}_4\text{Cl}$  solution is attributed to the lower hydration enthalpy of  $\text{NH}_4^+$  compared to that of  $\text{Na}^+$  (Peng *et al.* 2020; Morida *et al.* 2023). Hydration enthalpy serves as a measure of the interaction strength between a cation and water. Bentonite typically exhibits smaller swelling deformation in solutions with cations that have lower hydration enthalpy (Xiang *et al.* 2020). This results in differences in the swelling of bentonite in NaCl and  $\text{NH}_4\text{Cl}$  solutions. This indicates that the bentonite barrier may experience increased leachate percolation in  $\text{NH}_4\text{Cl}$  solution, potentially posing a risk of leakage for contaminants not adsorbed by the bentonite.

## 5. CONCLUSIONS

Permeation tests, concentration analysis, SI tests, and FTIR spectroscopy analysis were conducted to understand the pollutant interception and permeation characteristics of bentonite–natural clay mixtures in NaCl and NH<sub>4</sub>Cl solutions. It can be concluded from the following points.

- (1) The NH<sub>4</sub><sup>+</sup> concentrations in leachate significantly decrease with increasing bentonite content. The reduction in the concentration can be as high as 99% compared with NH<sub>4</sub><sup>+</sup> initial concentration, indicating an effective containment of NH<sub>4</sub><sup>+</sup> in leachate. In the residual leachate concentration, the Na<sup>+</sup> concentration is higher than the NH<sub>4</sub><sup>+</sup> concentration, with the difference reaching up to 1,000 times at certain points. The results indicate that the bentonite–clay mixture intercepts NH<sub>4</sub><sup>+</sup> more effectively than Na<sup>+</sup>.
- (2) The hydraulic conductivity coefficient of mixed soil samples increases with the concentration of NaCl and NH<sub>4</sub>Cl solutions. The mixed soil samples demonstrate different hydraulic conductivity characteristics in NaCl and NH<sub>4</sub>Cl solutions. Compared to NaCl solution, the samples have a lower SI and a higher hydraulic conductivity coefficient in NH<sub>4</sub>Cl solution. When a solution with a concentration of 74.8 mmol/L was used to permeate a mixed sample with 5% bentonite content, the difference in permeability coefficient reached 1.5 times.
- (3) NH<sub>4</sub><sup>+</sup> is more readily intercepted in the interlayers of montmorillonite through cation exchange than Na<sup>+</sup>. Additionally, the smaller hydration radius of NH<sub>4</sub><sup>+</sup> and the formation of hydrogen bonds contribute to the lower concentration of NH<sub>4</sub><sup>+</sup> compared to Na<sup>+</sup> in the leachate. This indicates that the bentonite barrier exhibits better interception of NH<sub>4</sub><sup>+</sup>.
- (4) The variation in hydraulic conductivity coefficient can be explained by the double-layer theory. Positively charged cations weaken the repulsion between the negative charges in the layers, causing layer compression. This leads to an increase in the gaps between particles, consequently increasing the hydraulic conductivity of the samples. NH<sub>4</sub><sup>+</sup> has a lower hydration enthalpy than Na<sup>+</sup>, and bentonite typically exhibits smaller swelling deformation in solutions with cations of lower hydration enthalpy. This results in differences in the hydraulic conductivity coefficient of the samples in NaCl and NH<sub>4</sub>Cl solutions.

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## ETHICS STATEMENT

Free and informed consent of the participants or their legal representatives was obtained and the study protocol was approved by the Donghua University.

## DATA AVAILABILITY STATEMENT

Data cannot be made publicly available; readers should contact the corresponding author for details.

## CONFLICT OF INTEREST

The authors declare there is no conflict.

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